PRELIMINARY GEOTECHNICAL INVESTIGATION REPORT COWBOY HEAVEN DEVELOPMENT PHASE 3-C: Areas III, IV, and V MOONLIGHT BASIN RANCH BIG SKY, MONTANA

Prepared for:

Moonlight Basin Ranch P.O. Box 1369 Ennis, Montana 59729

February 8, 2007



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Geotechnical Engineering

February 8, 2007

Lee Poole Moonlight Basin Ranch P.O. Box 1369 Ennis, Montana 59729

RE: GEOTECHNICAL INVESTIGATION, COWBOY HEAVEN PHASE 3-C, AREAS III, IV, AND V, MOONLIGHT BASIN RANCH, BIG SKY, MONTANA.

Dear Lee:

We are pleased to present this report on our geotechnical investigation for the Cowboy Heaven Phase 3-C Development Areas III, IV, and V, Moonlight Basin Ranch, Madison County, Montana. Two copies are enclosed. The report describes site conditions and presents conclusions and recommendations.

If you have any questions about this report, or if we may provide other services to you, please contact us. As the project progresses, we will be available to answer questions for you.

Respectfully submitted

Ray Womack Enclosure: Repo

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EXECUTIVE SUMMARY

The purpose of this investigation was to evaluate subsurface conditions for three specific areas of the Cowboy Heaven development Phase 3-C: 1) Area III 2) Area IV 3) Area V. The geologic map has also been updated to reflect results of other investigations performed at Cowboy Heaven over the past few years. Five test pits were excavated to investigate soil and groundwater conditions. The Cowboy Heaven development area is situated primarily on "dip slopes" on the north side of Lone Mountain. A relatively small area along the west edge of the subdivision is located on ancient dormant landslides (Exhibit 1). The rest of the proposed development area is located on ground that appears to be stable under current conditions. Subsurface conditions are variable even in the mapped areas, and site-specific subsurface investigations are strongly recommended for individual building sites throughout the development area. Limited site access at the time of the investigation precluded detailed exploration. This report is believed to be adequate for planning and subdivision design. Detailed foundation design for buildings is likely to require lot-specific exploration.

Of the five test pits, only one encountered material that could be considered suspect with regard to previous mass movement. The majority of the test pits encountered bedrock that was either flat-lying or inclined with a sub-perpendicular orientation relative to the local slope. Areas IV and V are underlain by coherent sandstone and weathered claystone bedrock. Area III is underlain by northward-dipping shale, claystone, and bentonite with some evidence of shearing apparent in the bentonite. The sheared bentonite is near the surface and can be addressed by local over-excavation of foundation cuts.

Areas IV and V that are underlain by sandstone bedrock appear to be acceptable for development using standard construction methods. For areas underlain by shallow bedrock with bentonite beds, some modification to foundation design and subgrade improvement may be required depending on local site conditions. In most cases the selected technique will probably be to "key" the foundation through the bentonitic clay layers into the underlying bedrock, using relatively short drilled piers or engineered fill to replace the soil. Multiple bentonite beds often occur in the bedrock, and it is important to verify that all shear zones and shallow bentonite beds are penetrated by the foundation elements.

1.0 INTRODUCTION

As authorized by Moonlight Basin Ranch, Womack & Associates, Inc. conducted a preliminary geotechnical investigation for the proposed Cowboy Heaven Phase 3-C Development, Areas III, IV, and V in Section 24, T6S, R2E, Madison County, Montana. The scope of services included reconnaissance level field mapping, and logging and sampling of 5 test pits. The purposes of this investigation are to verify the thickness and consistency of potential landslide deposits observed during previous investigations, document subsurface soil conditions in previously unexplored areas, evaluate soil-engineering properties, and provide preliminary recommendations for site development. Considering the variability in subsurface conditions and gaps between exploration points, the report should be used for planning and preliminary design purposes only. Site-specific geotechnical investigations will be required for final design of individual structures. This work complements the previous investigation undertaken for Cowboy Heaven Phase 3A to the west.

This investigation targeted only specific areas of the overall development in Section 24. A large portion of Section 24 has been evaluated during previous geotechnical investigations including the geotechnical investigations for Moonlight Lodge and Saddle Ridge Townhouses (Womack & Associates, 1996, 1998, and 1999), preliminary geologic hazards evaluation from aerial photographs for a large area of Moonlight Basin (Womack & Associates, 1998), preliminary investigation of Cowboy Heaven Phase 2 and 3 (Womack & Associates, 2001), and preliminary investigation of Cowboy Heaven Phase 3 (Womack & Associates, 2002). Where appropriate, subsurface information, laboratory test results, and geologic interpretations from the previous investigations are incorporated into this report; however, the previous studies should be referenced for conclusions and recommendation for areas outside the specific locations addressed in this report. Exhibit 1 represents a compilation of geologic interpretations from our current and previous investigations and supercedes all previous versions of this map.

2.0 PROPOSED CONSTRUCTION

According to the preliminary plat for Cowboy Heaven Phase 3-C, Area III is slated for development as single-family homesites and condominiums. There do not appear to be plans on the current plat for development of Areas IV and V. Specific plans and detailed foundation designs are unavailable. Depending of the subsurface conditions on each lot, foundation elements may include perimeter spread footings with stem walls and concrete slab-on-grade floors, reinforced concrete mat foundations, or deep foundations such as helical or drilled piers. Foundation subgrade improvement and remedial slope stabilization measures may be required on some lots prior to construction. Coherent sandstone and claystone bedrock underlie much of the area, but the limited presence of landslide debris and shale bedrock with weak bentonitic clay layers indicates that the proposed homesites should be located and designed to cater to specific site conditions.

3.0 INVESTIGATION PROCEDURE

3.1 Analysis of Aerial Photographs

Recent (August, 1994; October, 1998) large format black-and-white aerial photographs were viewed stereographically to identify landslide features and glacial terrain. Geologic features identified on the air photos, refined using information from our reconnaissance field mapping and preliminary subsurface investigation, were transferred onto a topographic base map (Exhibit 1).

3.2 Field Investigation

Field work conducted on June 23, 2005, consisted of excavation and logging of five exploratory test pits. Data from earlier reports were utilized where appropriate. A preliminary geotechnical investigation for Cowboy Heaven was addressed in a report by Womack & Associates in September 2001. The test pits were spread across the three areas, with two in Area III, one in Area IV, and two in Area V. The test pits were excavated to practical refusal or to the reach limit of the trackhoe at depths ranging from 8 to 12 feet. The excavation equipment was provided by Moonlight Basin Ranch. Test pit locations are shown on Exhibit 1. Refer to Appendix B for copies of logs from earlier reports.

Soil types, thicknesses, and consistencies were observed and documented by an Engineering Geologist. Field conditions were described for all soil and rock types observed and representative samples were obtained from the soil horizons. Note that site conditions are highly variable and actual soil conditions across the site will differ from those represented in the borehole and test pit logs.

3.3 Laboratory Analysis

No laboratory tests were conducted for this investigation. Based our experience in the area, laboratory testing was judged unnecessary.

3.4 Report Preparation

The report presents results of air photo analysis and geologic mapping, subsurface exploration, engineering analysis, and recommendations for development and possible mitigation measures.

4.0 SITE CONDITIONS

4.1 Description

Areas III, IV, and V of the Cowboy Heaven Phase 3 Development are located on north and east-facing slopes of a convex hillside at elevations of 7,880' to 8,000'. Local slope angles vary from 6° (Area V) to 35° (Area III). Current land cover includes a mix of open meadow and mature coniferous forest. No potential geologic concerns were identified at areas IV and V appear to be devoid of potential geologic concerns, as these areas are underlain, in the area tested, by coherent flat-lying sandstone and claystone bedrock within three to five feet of the ground surface. No evidence of shearing or other mass movement is apparent. Area III is not quite so straightforward, as our investigation here encountered a sheared bentonite zone within shale bedrock. This area is very steep, as well. No landslide deposits are apparent and coherent bedrock is within seven feet of the ground surface. Site-specific investigations will be required for new construction in Area III and buildings will likely need to be keyed to coherent bedrock underlying the bentonite.

4.2 Geology

The basin to the north of Cowboy Heaven is the topographic expression of the northwest-trending Lower Basin geological syncline (trough) that parallels the Spanish Peaks uplift to the northeast (Montagne, 1976). It has been glaciated and is covered in most places by a veneer of glacial till.

The Cowboy Heaven development area is underlain by thinly bedded to laminated shale of the Cretaceous Colorado Group (Bolm, 1969), possibly within the subdivision of the Group named the Albino Formation or in the undifferentiated shales and siltstones above the Albino. The Albino Formation is described as non-resistant claystone, shale, and sandstone, with siliceous ash layers that have weathered to bentonitic clay. Numerous bentonite beds have been encountered in the shale. These appear to have provided the zones of weakness that sheared when the landslides moved. Near the development area, the sedimentary rocks are generally inclined (dip) to the north at about 0 to 20 degrees, roughly parallel to the slope of the ground surface. The dip of the layered bedrock appears to increase locally to about 30 to 40 degrees toward Lone Mountain to the south.

The Albino Formation and other late Cretaceous age rocks in this region have been extensively intruded by andesitic volcanic rocks of the Tertiary-age Lone Mountain igneous intrusive (Swanson, 1950). The andesite volcanic rock has invaded the older shales, and is referred to as a "Christmas tree laccolith", which in cross-section has branching vaults resembling the limbs of a tree. On the slopes north of Lone Mountain, layers of andesite have been injected along the bedding of the shale, and rocks believed to be andesite were observed in contact with shale in several test pits excavated in 1999 south of about elevation 8,240 (Exhibit 1). The intrusion dilates the shale, such that the dip increases closer to Lone Mountain and decreases downslope.

4.3 Landslides

Kehew, et al (1971) and Montagne (1976) did not map the large landslides on the north slopes of Lone Mountain. A preliminary map of the landslides prepared by Womack & Associates, Inc. (1998) included a slide lobe in the western portion of the proposed development area. The geomorphic expressions of the landslide deposits include hummocky topography, closed depressions, ponds, and small head scarps (slope breaks). The approximate locations of the landslides at Cowboy Heaven Phases 2 through 4 are shown on Exhibit 1.

The landslide deposits appear to be relatively old (perhaps thousands of years) and inactive. The ages of the landslides are unknown. Bailey (1971) observed that many landslide failures in this region of the northern Rocky Mountains were caused by an increase in available moisture and a higher water-table level in the Pleistocene (about 1.8 million years to 6,000 years before present). The landslide deposits at Cowboy Heaven are typical of large ancient landslide failures throughout the northern Rocky Mountains. Many of the landslides probably occurred immediately after the withdrawal of the ice following the last glacial period (approximately 10,000 years before present).

The landslides appear to consist predominantly of "dip slope failures"; i.e., sliding has occurred along weak beds in the shale, which is oriented approximately parallel to the slope. As discussed in Section 4.2 (Geology), the Albino Shale contains numerous bentonitic clay beds, and some of the test pits encountered one or two bentonite layers up to 3 feet thick. The test pits excavated during previous investigations that penetrated the slide deposits consistently encountered a sheared bentonite layer along the base of the debris that conforms to the bedding of the underlying rock. In some cases (e.g., TP01-9 and TP01-14, Womack & Associates, 2002), virtually intact layers of shale ("glide blocks") up to 7 feet thick had clearly been displaced. A "glide block" is a relatively intact section of rock mass that has moved downslope with translational displacement along a planar (bedding) failure surface. Therefore, encountering bedrock is not necessarily an indication that an excavation has reached the bottom of the landslide debris. No landslide terrain features were encountered in the preliminary investigation of Phase 3 Areas III, IV, and V.

4.4 Area III

Two test pits were excavated in the vicinity of Area III. Test Pit locations for test pits TP05-1 and TP05-2 are shown on Exhibit 1. Subsurface conditions are depicted on logs included in Appendix A. Test pits TP05-1 and TP05-2 were excavated to depths of 10 and 12 feet, respectively. Test pit TP05-1 encountered interbedded claystone and shale dipping north at an angle of about 38° (oriented perpendicular to the local ground surface, which faces east). Test pit TP05-2 encountered highly fractured shale bedrock with a single highly sheared bentonite bed. The shale was described as slightly moist, weak, weathered, and highly fractured. The bentonite bed was 0.7 to 3.5 feet thick and was described as slightly moist, light brown, highly plastic, highly fractured weathered claystone that appears

sheared from folding. The presence of sheared bentonite in this area indicates the need for further investigation in advance of construction. The orientation of the beds perpendicular to the local slope reduces the likelihood of landslide movements at this site. Aerial photo analysis did not indicate the presence of landslide features at Area III. Buildings at this site will likely need to be keyed into the shale below the bentonite layer, by means of over-excavation or pile systems.

4.5 Area IV

For this investigation, one test pit was excavated in the northern portion of Area IV (test pit TP05-5; Exhibit 1). The test pit encountered flat-lying strata with no indication of previous mass movement. The upper 4.8 feet of the soil column is composed of topsoil and residual claystone, with sandstone bedrock below to the bottom of the hole at 8 feet. The topsoil is 1 foot thick and was described as moist, black, soft silt with organic material and roots. The residual claystone is essentially a soil due to weathering processes and was described as light tan, soft to firm clay. The sandstone encountered from 4.8 feet to 5.4 feet was described as red brown and very strong with 2 to 6 inch joint spacing and 1 to 2 inch bedding. From 5.4 feet to the bottom of the hole at 8 feet, the sandstone was described as medium brown and weak with 6 to 8 inch joint spacing and 2 to 4 inch bedding. Test pit logs detailing site conditions are attached in Appendix A. Removal of the topsoil and residual claystone from below all foundation footings will likely be necessary for new construction. The sandstone bedrock, while logged as very weak rock, is still considerably stronger than most soils and will likely provide adequate bearing conditions in this area. Site-specific investigations are recommended for individual buildings in Area IV.

4.6 Area V

Two test pits were excavated in Area V, both to depths of 8 feet (TP05-3 and TP05-4; Exhibit 1). The test pits encountered flat-lying strata with no indication of previous mass movement. Test pit TP05-3 encountered coherent bedrock at 5 feet below the ground surface. The upper portion of the soil column in test pit TP05-3 was composed of 0.5 feet of topsoil underlain by 2 feet of colluvium, which in turn is underlain by 2.5 feet of claystone. The colluvium was described as moist, brown, soft, plastic clay with sand and gravel. The claystone was described as moist, light brown, weathered claystone with some sandstone in a soft clayey matrix. The colluvium and claystone contain too much plastic material be appropriate bearing layers for foundations. As a result, the upper 5 feet of the soil column will likely need to be removed below foundation footings down to the coherent sandstone bedrock below. TP05-4 is located directly upslope of TP05-3 and encountered bedrock at a depth of 3 feet with no colluvial cover, suggesting that colluvium thickness increases downslope.

Test Pit TP05-4 encountered strong sandstone bedrock at a depth of 3 feet, continuing to the bottom of the hole at 8 feet. The upper 3 feet of the soil column is composed of 0.5 feet of topsoil and 2.5 feet of weathered claystone. The weathered claystone was described as moist, orange/brown, soft, plastic clay with some 3-inch minus sand and gravel. The sandstone was described as moist, medium brown, strong, nearly horizontally bedded, jointed angular fragments with 10 inch maximum diameter. The weathered claystone should be removed from beneath foundation footings due to its high plastic content. The sandstone bedrock encountered at 3 feet will provide good bearing conditions. We recommend site-specific investigations for new construction in Area V. Detailed logs describing field conditions from this preliminary investigation are attached in Appendix A.

4.7 Groundwater

At the time of our investigation, groundwater was not encountered in any of the pits. The site investigation was performed during a time of year that is typically associated with the seasonal groundwater high. Seepage may be more widespread and shallow during wet years. Seasonal wet surface areas mapped by others are shown on Exhibit 1.

4.8 Earthquakes and Seismicity

A northwest trending earthquake zone known as the Intermountain Seismic Belt (Smith and Arabasz, 1991) crosses Western Montana. The seismic belt enters Montana in the south near the Yellowstone Park-Hebgen Lake area, trends northward to the vicinity of Bozeman, and continues through Helena and Kalispell. Big Sky and the proposed development are located in the center of the seismic zone; however, active faults have not been identified in the Big Sky basin and historically no major earthquake epicenters have been located in the immediate area.

A recent national seismicity study by the U.S. Geological Survey (USGS, 2002) estimated peak horizontal accelerations on bedrock of 0.21g and 0.40g for earthquake events predicted to have risks of occurrence of 10% and 2% respectively in 50 years.

Ground motion accelerations and a design response spectrum were derived for the project site in accordance with the procedure defined in the 2006 International Building Code (IBC). The provisions of the 2006 IBC are intended to provide uniform levels of performance for structures, depending on their occupancy and use and the risk inherent to their failure. The approach adopted in the 2006 IBC is intended to provide a uniform margin of safety against collapse at the *design* ground motion. The *design* earthquake ground motion is selected at a ground shaking level that is 2/3 of the *maximum considered earthquake* (MCE) ground motion, which has a likelihood of exceedance of 2 percent in 50 years (a return period of about 2,500 years). The Site Ground Motion and Design Response Spectra differ depending on the earth material being considered as a function of varying shear wave velocities. The site is classified based on the

upper 100 feet of the soil and bedrock column. Based our investigations and published geology of the area (Bolm, 1969), we assign Areas III, IV, and V to a Very Dense Soil and Soft Rock Profile (Site Class C). Design Response Spectra are presented in Figure 1.

Earthquake Loads - Site Ground Motion and Design Response Spectrum 2006 International Building Code (2)

Cowboy Heaven Phase 3: Areas III, IV, and V.

Approximate Site Location: Latitude = N 45.297°, Longitude = W 111.419°

Mapped Maximum Considered Earthquake (MCE) Spectral Response Acceleration Parameters (USGS 2002):

Short Period (S_s) = **0.97**⁽¹⁾

1-Sec Period $(S_1) = 0.31^{(1)}$

Site Class Definition: C - Very Dense Soil and Soft Rock Profile

Site Coefficients and Adjusted MCE Spectral Response Acceleration Parameters:

 $S_{MS} = 0.98$

 $F_a = 1.01$ *interpolated from table

 $S_{M1} = 0.46$

 $F_V = 1.49$ *interpolated from table

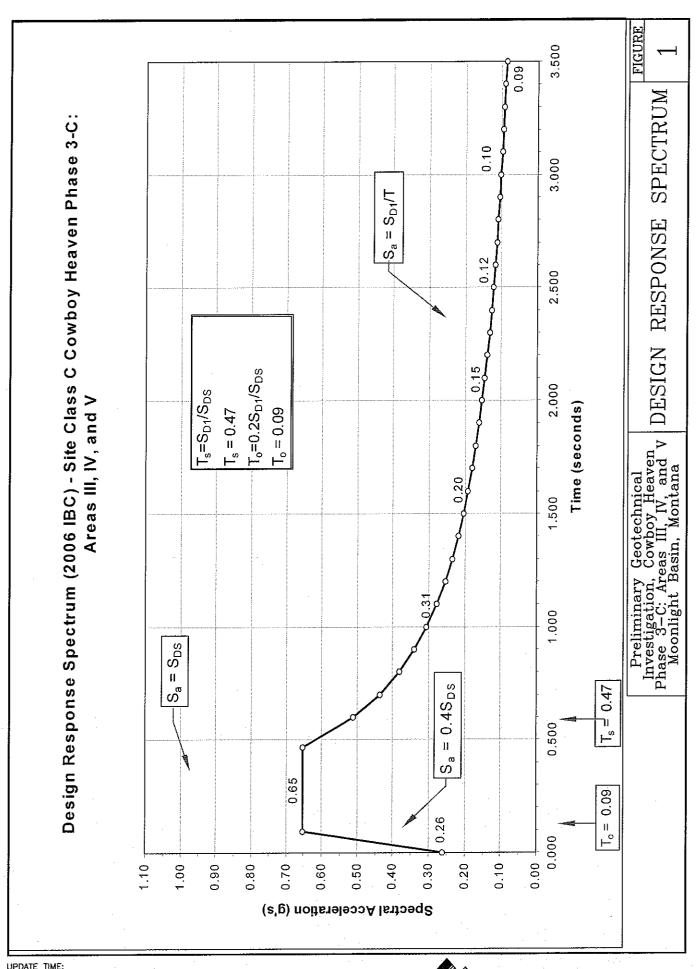
Design Spectral Response Acceleration Parameters

 $S_{DS}=0.65$

 $S_{D1} = 0.31$

⁽¹⁾ Probabilistic ground motion values (2% PE in 50 years) were obtained from the U.S.G.S web site; 2002 acceleration data.

^{(2) 2006} International Building Code, Chapter 16, Section 1615 - Earthquake Loads - Site Ground Motion.



5.0 ENGINEERING ANALYSIS

The engineering analyses are based on a limited number of sparsely distributed data points and are suitable for preliminary design only.

5.1 Consolidation/Swell Potential

No consolidation and swell testing was performed for this study. Laboratory test results from previous investigations indicate that the plastic clay samples observed in some of the test pits are expansive, but the high antecedent moisture content (25% or greater) may limit the swell potential. However, if significant drying of the soils occurs during construction, a high swell potential may result. Moderate consolidation may also be expected if footings are placed on soft plastic soil horizons. As shown on Exhibit 1 and test pit logs in Appendix A, a bentonitic clay bed was observed only in test pit TP05-2 in the northern portion of Area III, but may occur elsewhere. These materials are highly expansive and become very weak when wet.

5.2 Bearing Capacity

Bearing capacity of soil refers to its ability to resist shear failure under load. Preliminary bearing capacities for the site may be on the order of 3,000 psf for shallow foundation elements placed on the variable soil deposits and 5,000 psf if footings are placed on intact bedrock. Because the conditions are locally variable, we recommend evaluation of each building site for specific site conditions to support final design. Depending on the specific conditions at any given site, a variety of foundation designs may be appropriate. If weak or expansive materials are encountered at footing level, a replacement fill may be appropriate to key the foundations into the bedrock. Minimum footing depths should be designed to satisfy local codes and building requirements for prevention of frost heave. In some cases a deep foundation system may be preferable.

5.3 Lateral Pressures

Lateral pressures were calculated using methods suggested by Bowles (1996). Equivalent fluid pressures (γK) will vary depending on the soil types and the slope of the ground surface adjacent to foundation or retaining walls. Lateral pressure increases when the ground surface slopes toward the wall and vice versa. Lateral pressures were calculated for at rest, active, and passive conditions for level ground and sloping ground, assuming a surface slope adjacent to the foundation walls of about 11° (20%). Actual pressures will depend on the building location and final grading plans. Lateral pressures on retaining walls from earthquakes were estimated using the Mononobe-Okabe equations (Bowles, 1996; Duncan et al, 1990). Because the maximum acceleration occurs only briefly during an earthquake, it is common practice when designing dams and other earth structures to reduce the design acceleration to $\frac{1}{2}$ of the maximum design acceleration (Hynes and Franklin, 1984). We have calculated equivalent fluid pressures using a horizontal acceleration k_h of 0.10g (1/2 of k_h max).

Lateral pressure design parameters for variable conditions of slope, soil type, seismic conditions, and wall configurations are summarized in Table 5-1. If necessary, lateral pressures may be reduced by using imported granular material to backfill behind basement and retaining walls. The values in Table 5-1 have been calculated for the clay, sand, and gravel colluvial and residual claystone material that mantles much of Areas III, IV, and V.

For basement walls or other walls not allowed to deflect, lateral pressure design should utilize at-rest pressures (γK_o) or active seismic pressures (γK_{ae}) , whichever is greater. For retaining walls or other walls allowed to deflect and develop a full active soil wedge, use active seismic pressures (γK_{ae}) .

TABLE 5-1: Lateral Pressure Parameters

Condition	Coefficient of Earth Pressure*	γK (equivalent fluid pressure) ⁺				
Static Conditions						
Level Backfill	$K_0 = 0.53$	61 pcf				
	$K_a = 0.36$	42 pcf				
	$K_p = 2.77$	319 pcf				
Slope above (11°)	$K_{o} = 0.66$	75 pcf				
	$K_a = 0.41$	47 pcf				
Slope below (-11°)	$K_p = 1.99$	229 pcf				
Earthquake Conditions						
Level Backfill	$K_{ae} = 0.43$	49 pcf				
Slope above (11°)	$K_{ae} = 0.51$	58 pcf				
Level Backfill	$K_{pe} = 2.60$	299 pcf				
Slope below (-11°)	$K_{pe} = 1.80$	207 pcf				

^{*} ϕ equal to 28°

Should the site conditions for a specific site exceed the slope gradients used in the table, this office should be contacted and appropriate values should be calculated.

5.3.1 Active Pressures

For lateral pressure design of retaining walls, which are allowed to deflect and develop an active soil wedge, the calculated equivalent fluid pressure (γK_a) is about 42 pcf (pounds per cubic foot), assuming a horizontal ground surface

⁺ γ equal to 115 pcf

behind the retaining wall, or about 47 pcf for an 11-degree slope above the wall. This pressure distribution would be equivalent to a force of approximately $21H^2$ pounds per horizontal foot of wall acting at one-third the wall height (H) above the base, or $23.5H^2$ for sloping ground above the wall.

Research has indicated that lateral pressures due to earthquakes are non-hydrostatic in distribution, and the resultant acts above the lower third-point of the wall (Bakeer, et al, 1990). Accordingly, active soil pressures have been divided into two components that act at different wall heights. The static force acts at the lower third-point, as discussed above. The Mononobe-Okabe equations are often used to estimate dynamic forces against retaining walls. Although there is considerable debate about the theoretical applicability of these equations to rigid walls, they have been used for many years for seismic design and the performance record of underground walls during earthquakes has generally been good. The dynamic component of force is estimated as $\frac{1}{2}$ the difference of γK_{ae} - γK_a . This force would be in addition to static active earth pressure, and is equivalent to $3.5H^2$ and $5.5H^2$ pounds (for both flat and 11-degree slopes, respectively) per horizontal foot of wall applied at 50% of the wall height above the base.

5.3.2 Passive Pressures

For passive pressure design, the earth pressure coefficient (γK_p) varies from about 319 pcf, assuming a horizontal ground surface adjacent to the wall, down to 207 pcf for a negative 11-degree slope below the wall and seismic coefficient of 0.10g.

5.3.3 At-Rest Pressures

For lateral pressure design of basement walls, which are restrained and not allowed to deflect, the calculated at rest earth pressure (γK_o) is about 61 pcf, assuming a horizontal ground surface behind the basement wall, or 75 pcf for an 11-degree slope toward the wall.

5.5 Soil Friction

Terzaghi, et al (1996) suggests use of 30 degrees for the maximum friction angle along a concrete base in granular soils. Accordingly, a friction value of 0.58, which is the tangent of 30 degrees, is suggested. The friction value may be combined with the passive pressure to resist horizontal loads.

6.0 RECOMMENDATIONS

Given that most of the investigation encountered flat-lying undisturbed bedrock, we believe that the majority of the site is stable in its current state and suitable for the proposed construction, although there is some risk that future conditions, such as a large earthquake, unusual precipitation, or manmade modifications, could initiate instability and new slope movement. The northern portion of Area III, where sheared bentonite was encountered, may require special construction techniques to key construction into coherent bedrock. It should also be noted that unfavorable foundation conditions may exist locally across Areas III, IV, and V (such as additional plastic clay or bentonite layers below the depth of the test pits) that have not been addressed in this report. Site specific geotechnical investigations should be performed to verify soil and groundwater conditions at individual building sites prior to foundation design and construction.

6.1 General Siting Recommendations

The following recommendations are provided to reduce risk of damage resulting from conditions discovered during the site investigation. The main potential problem is slope instability.

- 1. Where possible, new construction should avoid slopes steeper than 25°.
- 2. New construction should avoid mapped landslide deposits. No landslide deposits were encountered in Areas III, IV, and V.
- 3. Where possible in areas with shallow bedrock with bentonite beds and on thin landslide deposits, the foundation should be keyed into coherent bedrock (A structural replacement fill may be required).

6.2 Foundations

In most of the Phase 3-C Areas III, IV, and V of Cowboy Heaven, conventional shallow foundations appear to be acceptable. However, the buildings should be founded on bedrock in order to decrease the risk of slope instability. Replacement structural fills may be used in thin surface colluvial deposits and areas of shallow bedrock with bentonite beds to key foundations into bedrock. Otherwise, drilled piers or similar systems are recommended.

6.3 Site Grading and Preparation

Properly compacted backfill and good site drainage are extremely important. Structural fill should consist of imported granular fill placed in lifts no greater than 8 inches loose thickness and compacted. Compaction specifications for structural fill, exterior backfill, and utility trench backfill are presented in the following table. Existing site material may be used for exterior and utility trench backfill:

Table 6-1: Compaction Parameters

Material Type	Minimum Compaction (%) ¹	Moisture Content $(\%)^2$
Structural Fill	95	± 3
Exterior Backfill	90	-1 to +3
Utility Trench Backfill	90	-1 to +3

¹ Compaction specification based on percent of maximum dry density according to ASTM D-698 (standard proctor test).

Structural fill should be used beneath exterior slabs-on-grade or to key foundation elements into bedrock. Do not overcompact exterior backfills against "green" foundation walls. Cohesive site materials should be used in the upper 2 feet of the exterior backfill to provide a lower permeability cap.

Prior to placement of structural fill for exterior slabs-on-grade, the site should be cleared and grubbed. No brush, roots, sod, frozen material, or other deleterious or unsuitable materials shall be incorporated in the foundation subgrade or structural fill.

Final grading should provide positive drainage of at least 1 foot in the first 12 feet away from the structure. Adequate gutters are strongly recommended. Roof runoff should be discharged at least 3 feet away from the building or exterior slabs. Swales or other moisture collection points should be avoided within 20 feet of the footings.

OSHA regulations (29CFR1926) appear to classify the soil at the site as Type C based on the existence of layers that dip into potential excavations at slopes steeper than 4H:1V. Therefore, temporary cut slopes should be no steeper than 1.5H:1V for excavations less than 20 feet in height. A trench box may be used if necessary to reduce trench excavation during utility installation. The contractor shall be responsible for adherence to OSHA and other safety regulations.

6.4 Foundation Drainage

The bentonite and weathered claystone materials commonly develop perched water layers that could cause seepage into basements. For this reason, footing drains are strongly recommended. Two potential drainage alternatives are suggested, as illustrated on Figure 2.

The least expensive technique would probably be a prefabricated composite drain. The composite drain consists of an open wick layer laminated to filter fabric to reduce infiltration of soil. The exterior of the wall is waterproofed, and the drain is laid against the waterproofing layer. The excavation is backfilled with compacted site material, as discussed in section 6.2, and the drain is covered by at least 2 feet of compacted site soil, sloped to drain (minimum

² Moisture content specification based on percent moisture variation from the optimum moisture content (OMC) determined according to ASTM D-698.

8%). The composite drain is wrapped around a perforated drainpipe at footing level, at least one foot below basement floor level. Prefabricated "socks" are available to facilitate the connection. The drainpipe may slope at a minimum 0.5%, and drain to daylight or a sump. If daylighted, the pipe should be protected with a flap gate to keep out rodents. Do not place a wire screen over the end of the pipe.

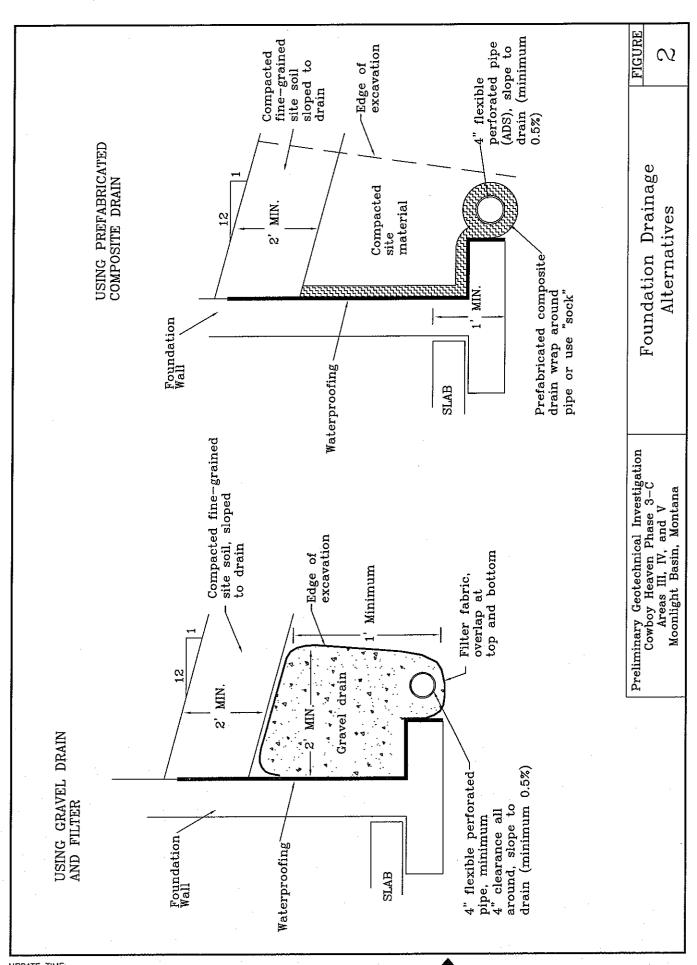
An alternative technique would involve placement of clean granular drain gravel between the foundation wall and the edge of the excavation, or between the excavation bracing and the wall. The gravel backfill is wrapped in filter fabric and a drain pipe is placed at the bottom of the trench. At least 2 feet of compacted clay backfill (sloped to drain) is placed above the gravel envelope. The gravel backfill can usually be placed without compaction, reducing backfill cost and difficulty. Also, gravel backfill may reduce the lateral stresses against the walls.

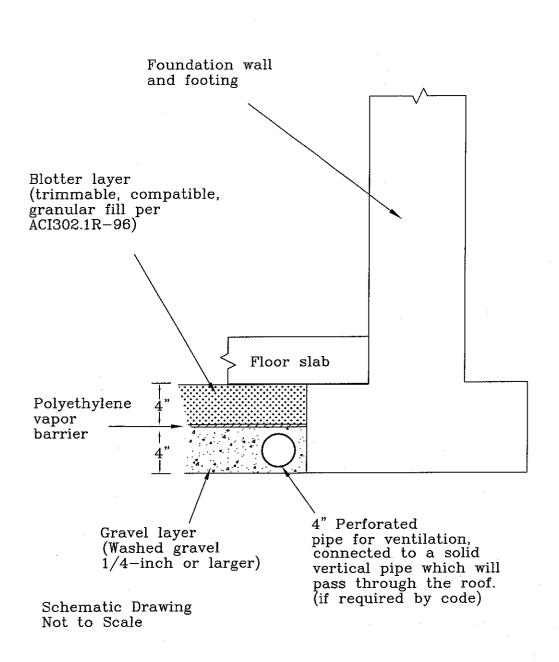
Site drainage is very important during foundation construction. Wetting of foundation soils can result in soil swell or consolidation/settlement, as well as reduction of shear strength and bearing capacity. Temporary drainage elements such as berms may be required during construction, depending on site conditions. If seepage is encountered, this office may be contacted to recommend appropriate action.

6.5 Interior Slabs-on-Grade

Interior slabs should be at least 4 inches thick, and any slabs bearing vehicles should be at least 6 inches thick. Minor floor cracking of slab-on-grade construction is difficult if not impossible to prevent. Such cracking is normal and should be expected to occur with time. Buildings are almost never free of cracks, and many factors other than soil movement, such as concrete shrinkage and daily and seasonal variability in temperature and humidity cause cracking.

An impermeable layer (usually plastic) is suggested beneath the slab, underlain by 4 inches of clean drain gravel which will act as a capillary break to reduce dampness (Figure 3). Two options are available to reduce the tendency





Preliminary Geotechnical Investigation Cowboy Heaven Phase 3-C Areas III, IV, and V Moonlight Basin, Montana

Interior Slab-on-Grade Ventilation Detail FIGURE

3

for the concrete to crack as it dries. Three articles from the American Concrete Institute (ACI) that discuss these options are appended.

- 1. A blotter layer may be placed under the slab. In the past, loose sand has been used for this purpose, but is no longer recommended. A cover of 4 inches of trimmable, compactible, granular material may be placed over the sheeting to receive the concrete slab. This material usually consists of "crusher run material", which varies in size from about 1.5-inch down to rock dust. Alternatively, 3 inches of fine graded material such as crusher fines or manufactured sand may be used.
- 2. The blotter layer may be eliminated if the concrete is reinforced properly. The attached article entitled "Controlling Curling and Cracking in Floors to Receive Coverings" provides a discussion of proper floor slab reinforcement. If the contractor needs additional guidance on reinforcement, a structural engineer should provide it.

6.6 Exterior Slabs-on-Grade

Exterior slabs (sidewalks, driveways, etc.) have typically sustained the greatest damage in areas such as Big Sky that are subject to frost heave and high seasonal temperature and moisture variability. As is the case with interior slabs, cracking of exterior slabs is almost impossible to avoid. The following suggestions may reduce differential movement of exterior slabs or pavement. The owners and developers should be aware that prevention of frost heave involving exterior slabs or roadways may require treatment of frost-susceptible soils to the full depth of frost penetration. Local codes typically stipulate a frost depth of 42 inches. However, experience has shown that frost penetration under roadways and slabs that have no snow cover may be much greater, on the order of 60 inches or more.

Exterior slabs should be at least 4 inches thick, 6 inches if supporting vehicles. Exterior slabs should not be tied to foundation walls. Any movement of exterior slabs may be transmitted to the foundation walls, resulting in damage. Posts for patios or other exterior columns should not bear on exterior slabs. If the slabs move, the movement is transmitted to the post, resulting in damage to the structure.

Exterior slabs may be underlain by at least 6 inches, preferably 12 inches, of gravel, sloped to drain to the outside, as illustrated on Figure 4. The gravel drain layer should extend at least 2 feet outside the edge of the slab, and be covered with cohesive site material. Perforated flexible CPE drainpipe may be placed along one or more edges, depending upon final grading. The pipe may be daylighted downslope, if possible. Expansion joints are recommended in all concrete flatwork.

Exterior slab, slope to drain

Fine-grained site material, sloped to drain

6" Min.

FINE GRAINED SOILS

6-12 inches of clean gravel, sloped to drain to the outside

-Filter fabric, lapped over top

4" perforated flexible CPE pipe, minimum slope 0.5%, minimum 4" gravel thickness around pipe

Schematic Drawing Not to Scale

Preliminary Geotechnical Investigation Cowboy Heaven Phase 3-C Areas III, IV, and V Moonlight Basin, Montana

Exterior Slab-on-Grade Drainage Detail FIGURE

4

12

6.7 Ventilation and Radon

Many building codes require that slabs below living spaces be ventilated to reduce the risk of radon infiltration. Ventilation should also be provided for areas under crawl spaces. Examination of the site for potential radon levels was beyond the scope of this report. If the owner wishes to pursue this matter, we can recommend appropriate contractors.

6.8 Reinforcing, Utilities Testing, and Concrete Considerations

Footings, slabs, and foundation walls should be reinforced to resist differential movement. Consultation with a structural engineer to specify adequate reinforcement is suggested. We recommend pressure testing of water and sewer lines before backfilling. Exterior concrete should contain 5% to 7% entrained air. We strongly recommend use of sulfate-resistant concrete.

6.9 Observation during Construction

A representative of this office should observe construction of any foundation or drainage elements recommended in this report. This recommendation is extremely critical for excavation of bentonite layers and evaluation of suitable foundation conditions. Structural fill, site grading, leak-proof testing, and soil compaction should be observed by a representative of this office. If any suspicious soils or conditions are revealed during construction, this office should be notified immediately to survey the conditions and make necessary modifications.

7.0 LIMITATIONS

This report has been prepared based on a limited amount of data and is intended for single use. Data points are scattered and additional exploration is required for final design of structures. The report and accompanying figures are not to be presented separately. Actual site conditions may vary. These services have been performed in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing in this area under similar conditions. No other warranty is made or implied.

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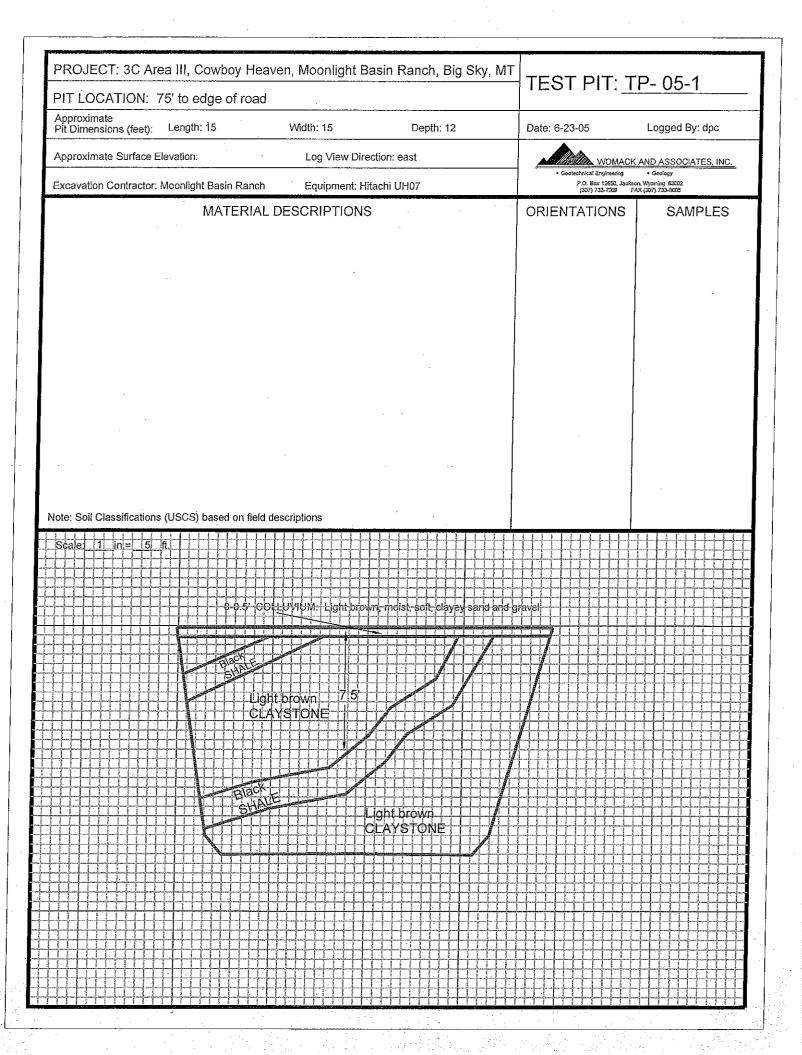
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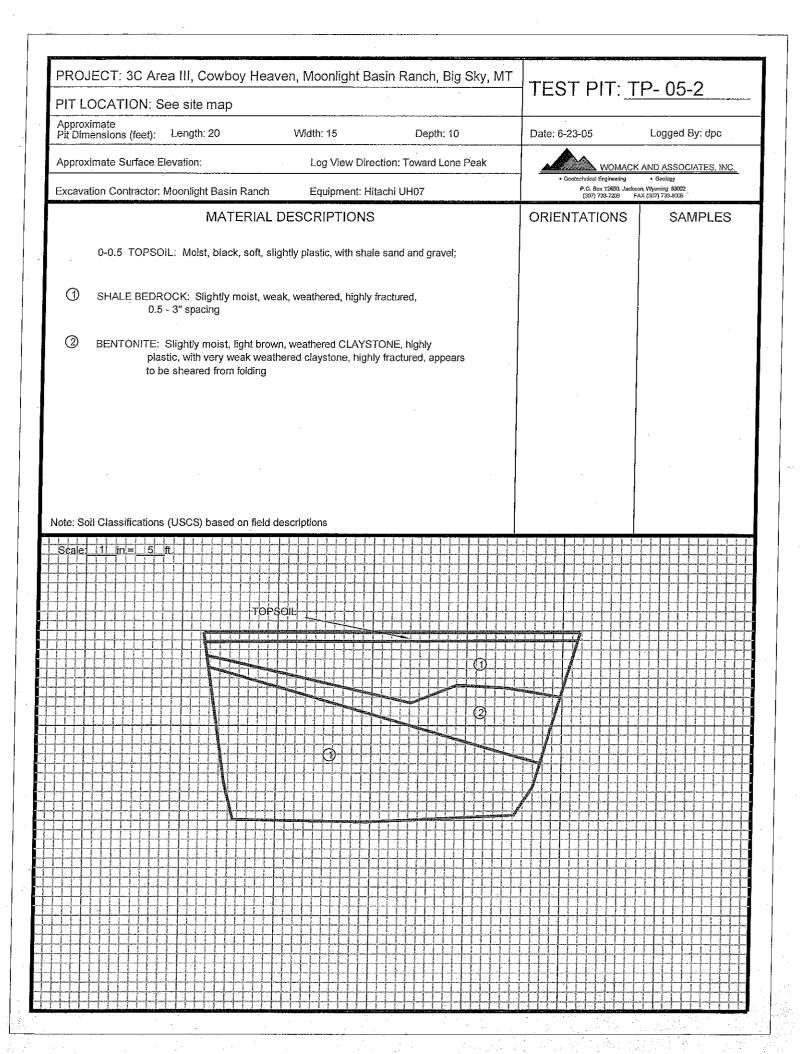
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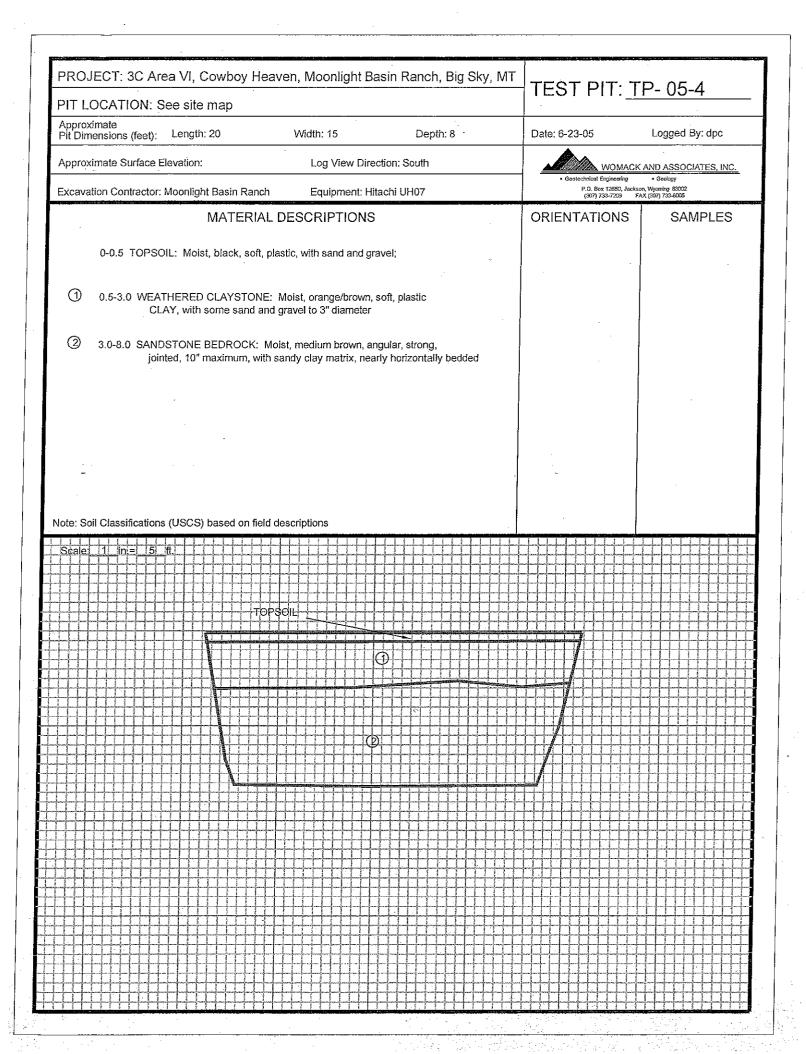
APPENDIX A

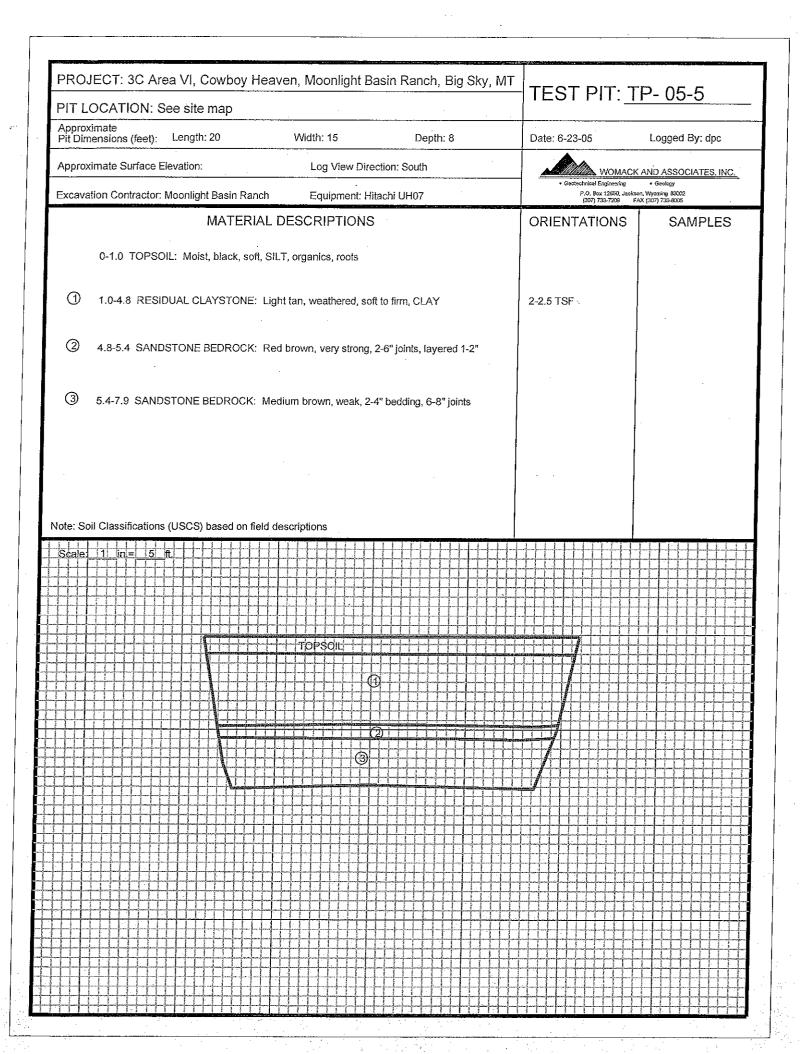
TEST PIT LOGS





PIT LO	OCATION: S	See site map)				TEST PIT: T	1-00-0
Approx Pit Dim	imate ensions (feet):	Length: 20	Width	: 15	Depth: 8	-	Date: 6-23-05	Logged By: dpc
Approx	imate Surface E	Elevation:	Lo	g View Direction: §	South		WOMACH • Geotechnical Engineering	AND ASSOCIATES, INC.
Excava	tion Contractor:	Moonlight Bas	in Ranch Eq	uipment: Hitachi U	H07		P.O. Box 12650, Jacks	
		MA	TERIAL DESCR	IPTIONS			ORIENTATIONS	SAMPLES
	0-0.5 TOPSC	DIL: Moist, blac	ck, soft, slightly plast	ic, with shale sand	and gravel;			
1	0.5-2,5 COLI sai	.UVIUM: Mois: nd and gravel	t, brown, soft, plastic	CLAY, with			·	
2		STONE: Mois ers, with clayey	t, light brown, weath y soft matrix	ered claystone, so	me sandstone	-		
3	5.0-8.0 SANE	STONE: Black	k, tan				,	
				,			¥	
oter So	il Classification	s (LISCS) base	d on field description	9				
Scale	1 in= 5	Int. I				<u> </u>		
			TOPSOIL					
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		Total Control				1 1		
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APPENDIX B

BOREHOLE AND TEST PIT LOGS FROM PREVIOUS INVESTIGATIONS

WOMACK & ASSOCIATES, INC. Geotechnical Engineering

TEST_HOLE_LOG MLBR GPJ WOMACK.GDT 12/15/98

TEST HOLE LOG

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PAGE 1 OF 1

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PROJ	JEC	T: N	/loonli	ght B	asin F	lanch						IOLE N	10.:7	P-98	3-1		ATE:	10-	16-98		
TEST	HO	LE L	OCAT	FION:	Wes	st port	tion of Are	ea 2, al	ong	main ro	ad										
ELEV	ATIO	ЭИС	3.S.:	-	ТС	TAL	DEPTH:	10.5	GF	ROUND'	WA	ER LE	VEL:	NA	MEA	ASU	RED	FRON	1 :		_
DRILL	U2 1.5- highly plastic, ma 1.75 5.3-6.5' SHALE:							<u> </u>		D	RILLE	R: CI	ayton		L	OGGE	ED BY	′: GS	 SV	-	
	ග				Te	z					!		• • • • • • • • • • • • • • • • • • • •		T			T			_
DEPTH (FT.)	GRAPHICAL	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TS	CLASSIFICATIO				DESCRI			:		FOLLA	GONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL	COMPLESS
1 - 1 - 2		U1					Topsoif 0.5-1.5' to slight! platy sha 1.5-2.7' highly pl 2.7-4.0' laminate 30°/10° w	Sandy S y moist, I ale gravel CLAY: E astic, ma SHALE: d, deeply vest	ILT w loose, I 1/2" Brown ssive Dark weat	ith gravel structure to 3", and to tan, sl [Bentonit brown, dr hered, ha	: Dar liess, i cobb ightly e/Res ry, ve ry, ve	k to med about 30 oles to 6" moist to sidual Be ry thinly laible, oris	dium bro 0% ango (Colluy moist, drock) bedded	own, dry ular rium] stiff,							
	l	J2	1.5- 1.75 1.5- 1.75 5.3-6.5' SHALE: Dark brown to thinly bedded, deeply weathered.						black	. drv. ver	v thiniv	to									
							\very think 6.7-10.5' thinky bed Fm]	/ bedded SHALE: ded, dee	, weal Dark ply we	k, very lov brown to eathered,	w hare black hard	dness k, dry, ve friable [ry thini Thermo	/ to polis							
				-					-			· .								12	

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TEST HOLE LOG

PAGE 1 OF 1

PROJECT: Moonlight Basin Ranch HOLE NO .: TP-98-7. DATE: 10-16-98 TEST HOLE LOCATION: South central portion of Area 2, ~50' north of road, ~225' west of powerline **ELEVATION G.S.:** TOTAL DEPTH: 13 GROUNDWATER LEVEL: NA MEASURED FROM: DRILL TYPE: Case extendahoe LOGGED BY: GSV DRILLER: Clayton LOG UNCONFINED STRENGTH (TSF) CLASSIFICATION DRY DENSITY (PCF) RECOVERY (%) WELL COMPLETION CONTENT (%) GRAPHICAL DEPTH (FT.) PLASTICITY INDEX (%) MOISTURE DESCRIPTION LIQUID LIMIT (%) SAMPLE 0.0-1.0' Silty CLAY: Very dark brown to black, moist, soft, roots, organics [Topsoil] 1.0-5.7' Silty CLAY: Dark brown to dark grayish brown (mottled), moist, medium stiff to stiff, massive to some stratification of 2" to 3" shale gravel layers [Colluvium] 1.0 U2 2.0 5.7-9.2' SHALE: Dark gray, slightly moist to wet at contact with colluvium, very thinly bedded to laminated, deeply weathered, fraible, low hardness, bedding appears to be 20° /10° west [Thermopolis Fm] Note: Bedding at shale/sandstone contact appears to be oriented more east-west with shallow dip to N (~5°) 9.2-13.0' Clayey to silty SANDSTONE: Light yellowish brown, slightly moist to moist, thinly bedded, moderately to deeply weathered, very low hardness, friable [Thermopolis 3

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PRO	DJEC	T: N	/loonli	ght B	asin R	anch			HOLE NO .: TP-91	 3-3	D.	ATE:	10-1	6-98	
TES	T HC	LE L	OCAT	ION:	Area	2, n	orth central Secti	ion 24, ~80-1	00' north of road, 150		n of	powe	rline		
ELE	VATI	ON C	3.S.:		ТС	TAL	DEPTH: 12.5	GROUNDW	ATER LEVEL: NA	МЕА	SUF	RED I	FROM	1:	
DRII	L TY	PE:	Case	exte	ndaho	e			DRILLER: Clayton		LC	GGE	D BY	: GS	V
DEPTH (FT.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION		DESCRIP		,	CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 3 3 4 5 6 7 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9							1.0-4.5' Silty CLA moist, medium sti slope, some roots fines, 30% fine to sandstone gravel 4.5-12.5' SHALE: moist, very thinly thardness, friable, 60°/11° N [Thermo	bris [Topsoil] AY with some sar iff to stiff, curde s and organic del coarse grained s to 3", scattered of Very dark brow bedded to lamina highly fractured, poolis Fm] I hardness vary fi	orown to black, moist, soft, and and gravel: Light brown stratification subparallel to pris, about 70% plastic sand and angular shale and cobbles to 8" [Colluvium] nish gray, slightly moist to sted, deeply weathered, low bedding 80°/14° N and from friable low hardness to deeply weathered.	, d					
3					-										

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TEST HOLE LOG

PAGE 1 OF 1

HOLE NO .: TP-98-4 PROJECT: Moonlight Basin Ranch DATE: 10-16-98 TEST HOLE LOCATION: Area 2 north central Section 24, ~450' west of TP-3, above obvious landslide scarp **ELEVATION G.S.:** TOTAL DEPTH: 13.5 GROUNDWATER LEVEL: NA MEASURED FROM: DRILL TYPE: Case extendahoe DRILLER: Clayton LOGGED BY: GSV GRAPHICAL LOG UNCONFINED STRENGTH (TSF) CLASSIFICATION DRY DENSITY (PCF) RECOVERY (%) CONTENT (%) WELL COMPLETION DEPTH (FT.) S.P.T. (N) BLOWS/FT MOISTURE DESCRIPTION LIQUID LIMIT (%) SAMPLE 0.0-1.5' Sandy SILT: Dark brown, moist, loose, roots [Topsoil/Reworked Fill] 1.5-6.5' Sandy CLAY with gravel and cobbles: Dark brown, slightly moist to moist, stiff to very stiff, massive, about 60% plastic fines, about 20% fine to coarse grained sand, and 20% angular to subrounded gravel and cobbles consisting of mostly sandstone, some andesite and few shale [Glacial Till or Colluvium] 6.5-13.5' Sandstone COBBLES with silty to clayey sand matrix: Clast supported, dark yellowish brown, moist, dense, structureless, about 80% hard sandstone cobbles to 12", 20% fine grained sand and fines. At 9.0' increase in moisture, pockets of water and increase in clay content in matrix, some subrounded gravel At 11.0' very hard sandstone cobbles, difficult digging Uncertain of soil origin, predominantly sandstone cobbles, clast supported, no visible bed forms or layering, probably landslide debris [Colluvium or Landslide Debris]



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TEST HOLE LOG

PAGE 1 OF 1

Geatechnical Engineering Geology

Geology								
PROJECT NAME: Moonlight Bas	sin Ranch			D,	ATE: 6	5/28/99		
PROJECT LOCATION: Cowboy	Heaven Cabins, Big Sky, Montan	a		н	OLE NO).: T P	-99-1	
TEST HOLE LOCATION: See loc	cation map in report							
ELEVATION G.S.: ~8295	TOTAL DEPTH: 12.7	GROUNDWATER LEVEL: NA	ME	ASURE	ED FRC	M:		
DRILL TYPE: Trackhoe	HAMMER:	DRILL CO: MBR	DRILLER	: Shac	d	LOG	GED B	Y: wrw
GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT. RECOVERY (%)	STRENGTH (TSF) CLASSIFICATION CSTANDON	DESCRIPTION		MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 - 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	scattered angular 0.8-8.4' GRAVEL andesite, up to 6-i orange, loose to n 8.4-10.3' GRAVEL 3-inch diameter, in brown, loose, with [SCREE] SM 10.3-10.7' Silty SA loose, intact, nume of organics [PALEC 10.7-12.3' GRAVE 2-inch diameter, oc sand, thin layer of s top [TILL] GC 12.3-12.7' WEATH stiff, clayey GRAVE	AND - moist, orange, medium dense, to subrounded gravels, roots [TOPSC and COBBLES - angular to subround nch diameter, in a matrix of very mois nedium dense, clayey sand [SCREE] a matrix of wet, dark gray to greenish minor clayey sand infilling, many void ND - wet, grayish brown to pale gray, rous gravels and roots, lenticular pate DSOL] L - angular to subrounded andesite, casional cobbles, in a wet, loose, clay stiff, orange, gravelly, plastic clay at the ERED ANDESITE - moist, plastic, vet. Weathered rock with clay in the fractic weathered rock with clay in the fractic	DILI ded st,	21.1		NP 30	NP 15	



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TEST HOLE LOG

PAGE 1 OF 1

Geotechnical Engineering Geology

HOLE_LOG2 MBRCH99.GPJ WOMACK.GDT 9/9/99

DATE: 6/28/99 PROJECT NAME: Moonlight Basin Ranch PROJECT LOCATION: Cowboy Heaven Cabins, Big Sky, Montana HOLE NO .: TP-99-2 TEST HOLE LOCATION: See location map in report ELEVATION G.S.: ~8284 TOTAL DEPTH: 10.4 GROUNDWATER LEVEL: perched @ MEASURED FROM: ground surface 4.5' DRILLER: Shad LOGGED BY: wrw DRILL TYPE: Trackhoe HAMMER: DRILL CO: MBR UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) MOISTURE CONTENT (%) DESCRIPTION WELL COMPLETION DEPTH (FT.) PLASTICITY INDEX (%) S.P.T. (N) BLOWS/FT. COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-0.2' SOD 0.2-0.6' Clayey SAND - moist, orange, medium dense, scattered, angular to subrounded gravels, roots [TOPSOIL]
0.6-4.5' GRAVEL and COBBLES - angular to subrounded andesite, in a matrix of moist, yellow brown, clayey, coarse-grained sand (60% Gravels) [LANDSLIDE DEBRIS] U1 0.6 CH 27.3 96.2 55 37 4.5-4.6' CLAY - very moist, reddish brown, firm, plastic, slickensided [SHEAR ZONE] U2 (shear zone dip ~10 degrees, dip direction ~ N47E) Isolated seepage along top in gully
4.6-7.1' DISTURBED SHALE - dark gray to grayish brown, very weak, highly fractured, distorted [LÁNDŠLIDE DEBRIS] 7.1-7.4' CLAY - moist, pale gray to yellowish orange, stiff, plastic, slickensided [SHEAR ZONE] UЗ CH 26.5 88.4 58 33 1.6 \(shear zone dip ~12 degrees east, strike N25W)
7.4-10.4' SHALE - black, very weak, laminated, sheared with Ų4 slickensides, bedding somewhat distorted and irregular, but apparently in place [BEDROCK] Perched water at ~8' Bedding orientation ~ due north; 30 degrees east



TEST_HOLE_LOG2 MBRCH99.GPJ WOMACK.GDT 9/9/99

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TEST HOLE LOG

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Geotechnical Engineering
Ceology

PROJECT NAME: Moonlight Basin Ranch

DATE: 6/28/99

ROJECT LOCATION: Cowboy	y Heaven Cabins, Big Sky, Montar	na		HOLE NO.	: TP-99-3	
EST HOLE LOCATION: See to	ocation map in report	· · · · · · · · · · · · · · · · · · ·				
LEVATION G.S.: ~8280	TOTAL DEPTH: 18.5	GROUNDWATER LEVEL: NA	MEAS	URED FROM	M:	
RILL TYPE: Trackhoe	HAMMER:	DRILL CO: MBR	DRILLER:	Shad	LOGGED BY:	: wrw
GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT. RECOVERY (%)	UNCONFINED STRENGTH (TSF) CLASSIFICATION OO OO STANDARY	DESCRIPTION	MOISTIBE	CONTENT (%) DRY DENSITY (PCF)	LIQUID LIMIT (%) PLASTICITY INDEX (%)	WELL
	numerous gravel 1.3-8.7' Clayey S dense, intact, coa to subrounded gr [COLLUVIUM] 8.7-18.5' GRAVE subrounded andes	ELAY - moist, yellowish brown, soft, in s and roots [TOPSOIL] SAND - moist, yellowish brown, mediu arse to fine-grained, with numerous ro avels, occasional andesite cobbles L and COBBLES - subangular to site, up to 6-inch diameter, in a matrix rown, stiff, sandy Clay [TILL]	m unded			



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PROJECT	NAME	: Moonl	ight B	asin Ra	nch			DATE: 6	5/28/99		
PROJECT I	LOCAT	rion: c	owbo	y Heave	n Cabi	ns, Big Sky, Montana		HOLE NO	D.: TP	-99-3	 -
DEPTH (FT.) GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	DESCRIPTION	MOISTURE	CONTENT (%) DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL
7											
9											



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TEST HOLE LOG

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PROJECT NAME: Moonlight Basin Ranch DATE: 6/28/99 PROJECT LOCATION: Cowboy Heaven Cabins, Big Sky, Montana HOLE NO.: TP-99-4 TEST HOLE LOCATION: See location map in report ELEVATION G.S.: ~8300 TOTAL DEPTH: 7 GROUNDWATER LEVEL: NA MEASURED FROM: DRILL TYPE: Trackhoe DRILLER: Shad HAMMER: DRILL CO: MBR LOGGED BY: wrw UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION MOISTURE CONTENT (%) WELL COMPLETION DEPTH (FT.) PLASTICITY INDEX (%) S.P.T. (N) BLOWS/FT. COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-0,3' SOD 0.3-2.5' Sandy CLAY - moist to very moist with depth, yellowish brown, stiff, intact, roots [TOPSOIL] 3.0 2.5-4.2' RESIDUAL ANDESITE - moist, pale yellowish gray, stiff, intact, plastic, slightly sandy CLAY, roots, possible shearing at base [BEDROCK] 2.8 4.2-7.0' ANDESITE - dark greenish gray, weak, highly fractured, banding ~1-inch thickness to very thin bedded near 4.9-foot depth [BEDROCK] Banding thickness about 2 to 6-inches Banding orientation (dip) - 8 degrees northeast



Geology

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TEST HOLE LOG

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PROJECT NAME: Moonlight Basin Ranch DATE: 6/28/99

ELEV	ATION	G.S.:	~8265	i	ТОТ	TAL DE	PTH: 14	GROUNDWATER LEVEL: NA		MEASUR	ED FRO	DM:		
DRILL	TYPE	: Tra	ckhoe		НАМ	MER:	· · ·	DRILL CO: MBR	DRILL	ER: Sha	ıd	LOG	GED E	3Y:
DEPTH (FT.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION		MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL
2 3 4 7 5 6 7 8 7 9 9 10 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		D1 ®			0.9	СН	7.8-9.5' CLAY - vemicroshattered, plands [GOUG] Orientation of sheat of the plands of	d SHALE - black to yellowish brown, deformed, very weak, slid into place BRIS] ery moist to wet, yellowish brown, firrastic, abundant shale and andesite E -SHEAR ZONE] ar zone - 37 degrees downslope TE - dark greenish brown, weak, ran ctured, with irregular rubblized zone mal shearing down to about 12.5' n (dip) - 8 degrees northeast	domly	23.3		59	38	
3														



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PRO	JECT	NAME.	: Moon	light B	asin Ra	ınch		-			D,	ATE:	6/28/99)	
PRO	JECT !	_OCA1	TION: C	owbo	y Heave	en Cab	ins, Big Sky, Montan	na			Н	OLE NO	D.: TP	-99-6	
TEST	HOLE	LOC	ATION:	See I	ocation	map ir	n report				'				
ELEV	ATION	I G.S.:	-8315	;	TO.	TAL DE	EPTH: 10.5	GROUNDWATER LEVEL: slow seepage @ 9'		MEA	SURE	D FRO	DM: gr	ound s	urface
DRIL	L TYPI	E: Tra	ackhoe		НАМ	MER:		DRILL CO: MBR	DRIL	LER:	Shad	t	LOG	GED E	BY: wrw
DEPTH (FT.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 1 2 3 3 4 4 5 5 6 6 6 7 8 8 9 9 110 111 112 113 113 113 113 113 113 113 113		D1 ®			1.0	CL	3.0-3.8' Sandy Clintact, roots and s 3.8-4.8' CLAY - m numerous shale fr Below 4.3' clay is Shear zone at 4.6' 4.8-5.8' CLAY - w stiff, microshattere 5.8-8.4' CLAY and brown, stiff, thinly	ery moist, dark gray to yellowish bro ed, plastic, with shale fragments [G0 d SHALE fragments - moist, yellowi laminated [GOUGE]	n, firm, RIS] h wwn, DUGE] sh		20.0		48	28	
4-															



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TEST HOLE LOG

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HOLE_LOG2 MBRCH99.GPJ WOMACK.GDT 9/9/99

PROJECT NAME: Moonlight Basin Ranch DATE: 6/28/99 PROJECT LOCATION: Cowboy Heaven Cabins, Big Sky, Montana HOLE NO.: TP-99-7 TEST HOLE LOCATION: See location map in report TOTAL DEPTH: 7.6 MEASURED FROM: ground surface ELEVATION G.S.: ~8285 GROUNDWATER LEVEL: 7.5 DRILLER: Shad DRILL TYPE: Trackhoe HAMMER: LOGGED BY: wrw DRILL CO: MBR UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION DRY DENSITY (PCF) RECOVERY (%) DESCRIPTION MOISTURE CONTENT (%) WELL COMPLETION DEPTH (FT.) S.P.T. (N) BLOWS/FT COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-0.4' SOD 0.4-0.7' Clayey SAND - wet, pale-yellowish brown, very loose, intact, scattered angular andesite gravels and boulders, roots [TOPSOIL]

0.7-4.6' CLAY - moist, pale yellowish brown, firm, plastic, shear zone probably at base [GOUGE/LANDSLIDE DEBRIS] 1.0 CH 19.6 D1 😗 53 34 4.6-7.6' ANDESITE - dark gray, randomly and highly fractured, with minor clay along fractures [BEDROCK]



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Geotechnical Engineering Geology

HOLE_LOG2 MBRCH99.GPJ WOMACK.GDT 9/9/99

PROJECT NAME: Moonlight Basin Ranch DATE: 6/28/99 PROJECT LOCATION: Cowboy Heaven Cabins, Big Sky, Montana HOLE NO.: TP-99-8 TEST HOLE LOCATION: See location map in report ELEVATION G.S.: ~8275 TOTAL DEPTH: 11.5 GROUNDWATER LEVEL: 11.3 MEASURED FROM: ground surface DRILL TYPE: Trackhoe HAMMER: DRILL CO: MBR DRILLER: Shad LOGGED BY: wrw UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) DEPTH (FT.) PLASTICITY INDEX (%) S.P.T. (N) BLOWS/FT. MOISTURE COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-1.1' Sandy CLAY - moist, yellowish brown, firm, fissured, scattered gravels, roots [TOPSOIL] 1.1-5.0' BOULDERS - composed of Siltstone, black shale debris near surface [RUBBLE/LANDSLIDE DEBRIS] °0 = 00 °0 = 00 00 0 °Õ∈ 0 00 00 00 5.0-6.6' CLAY - very moist, yellowish brown, firm, plastic [GOUGE] CH 1.3 27.3 95.5 68 46 U2 Orientation (dip) of gouge - 31 degrees northeast 6.6-7.0' SHALE fragments - very moist, yellow, stiff, in a matrix of plastic clay [GOUGE]
7.0-9.8' SHALE - yellowish brown, very weak, laminated [LANDSLIDE DEBRIS] Orientation - north 5 degrees east, 28 degrees east ~9.6-9.8' CLAY - very moist, yellowish over maroon, firm, plastic [BEDROCK/SHEAR CONTACT] 1.0 9.8-11.5' ANDESITE - greenish gray, moderately strong, randomly and moderately fractured [BEDROCK]



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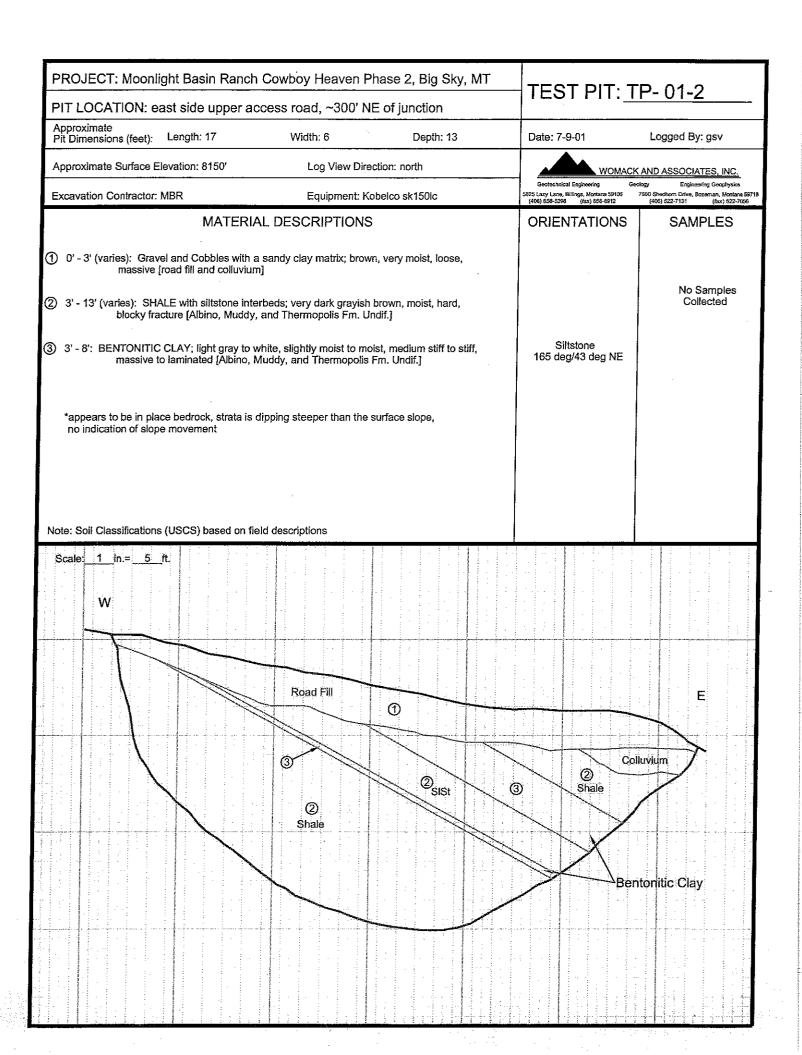
TEST HOLE LOG

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Geotechnical Engineering Geology

TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

PRO	JECT I	NAME:	: Moon	light B	asin Ra	nch					D.	ATE:	7-9-01		
PRO	JECT I	_OCAT	TON: C	owbo	y Heave	en, Pha	ase 2				H	OLE NO).: TP	-01-1	
TEST	Γ HOLE	ELOC	ATION:	Uppe	r acces	s road,	, 150' northeast from	junction							
ELEV	/OITA	1 G.S.	(Ft.): 8	164	то	ΓAL DE	EPTH (Ft.): 17	GROUNDWATER LEVEL (Ft.):	NA	MEA	SURE	ED FRO	DM:		
DRIL	L TYPE	≣: Kol	belco Sh	(150L	с намі	MER:		DRILL CO: MBR	DRIL	LER:	Clay	ton	LOG	GED B	Y: gsv
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT,	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1		P1-10					Light reddish brov 30-40% angular a to angular gravel 10-15% plastic cla vellowish brown, m clay, 20-25% fine to 1"	CLAY with gravel: Light reddish to loist, stiff, plastic, Imassive, about to coarse grained sity [GLACIAL DEPOSITS] CLAY with gravel: Light reddish to loist, stiff, plastic, Imassive, about to coarse grained sand, 20-25% graves are grained sand, 20-25% graves and COBBLES: As above, increbase	, about angular and,	c:					>
19															





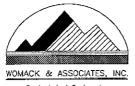
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TEST HOLE LOG

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TEST HOLE LOCATION: Present the limit of the location of the l	Phase 2 cabins, b	COMMENTS: 0.0-0.5' Sandy S loose, massive, c 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	GROUNDWATER LEVEL (FOR DRILL CO: MBR DESCRIPTION SILT: Dark brown, dry to slightly organic debris, scattered grave EL and COBBLES with clayey swn, slightly moist to moist, local about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS] Illapsing in upper 6'	DRIL DRIL If moist, If [TOPSOIL] sand matrix: se to medium	MEASU LER: CI	%) CF)	OM: LOG	GGED B	WELL AND THE STATE OF THE STATE
GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT.	50LC HAMMER:	COMMENTS: 0.0-0.5' Sandy S loose, massive, c 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	GROUNDWATER LEVEL (FOR DRILL CO: MBR DESCRIPTION SILT: Dark brown, dry to slightly organic debris, scattered grave EL and COBBLES with clayey swn, slightly moist to moist, local about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS]	DRIL DRIL If moist, If [TOPSOIL] sand matrix: se to medium	MOISTURE CO	ayton	LOG	۲ (
GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT.	50LC HAMMER:	0.0-0.5' Sandy S loose, massive, c 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	DRILL CO: MBR DESCRIPTION SILT: Dark brown, dry to slightlorganic debris, scattered grave EL and COBBLES with clayey wn, slightly moist to moist, local about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS]	DRIL DRIL If moist, If [TOPSOIL] sand matrix: se to medium	MOISTURE CO	ayton	LOG	۲ (
DEPTH (Ft.) GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT.		0.0-0.5' Sandy S loose, massive, o 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	DESCRIPTION SILT: Dark brown, dry to slightle organic debris, scattered grave EL and COBBLES with clayey own, slightly moist to moist, loos about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS]	ly moist, al [TOPSOIL] sand matrix: se to medium	MOISTURE			۲ (
GRAPHICAL SAMPLE S.P.T. (N) BLOWS/FT.	RECOVERY (%) UNCONFINED STRENGTH (TSF) CLASSIFICATION	0.0-0.5' Sandy S loose, massive, c 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	SILT: Dark brown, dry to slightl organic debris, scattered grave EL and COBBLES with clayey own, slightly moist to moist, loos about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS]	el [TOPSOIL] sand matrix: se to medium ingular gravel	MOISTURE	CONTENT (%) DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL
1		loose, massive, c 0.5-13.0' GRAVI Light reddish bro dense, massive, and cobbles to 8' plastic fines [GLA	organic debris, scattered grave EL and COBBLES with clayer iwn, slightly moist to moist, loo about 50-60% subangular to a ", 30% fine to coarse grained s ACIAL DEPOSITS]	el [TOPSOIL] sand matrix: se to medium ingular gravel	1				
10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0									



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TEST HOLE LOG

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Geotechnical Engineering Geology

TEST_HOLE_LOG2_MLBRCHP2.GPJ_WOMACK.GDT_8/9/01

Geology	•						
PROJECT NAME: Moonlight Bas	sin Ranch			ים	ATE: 7-	- 9- 01	
PROJECT LOCATION: Cowboy I	Heaven, Phase 2			н	OLE NO.	.: TP-01-4	
TEST HOLE LOCATION: Phase 2	2 cabins, central portion						
ELEVATION G.S. (Ft.): 8190	TOTAL DEPTH (Ft.): 14	GROUNDWATER LEVEL (Ft.): NA	А МЕ	EASURE	ED FROI	M:	
DRILL TYPE: Kobelco SK150LC	HAMMER:	DRILL CO: MBR	DRILLEF	R: Clay	ton	LOGGED	BY: gsv
DEPTH (Ft.) GRAPHICAL LOG SAMPLE S.P.T. (N) BLOWS/FT. RECOVERY (%)	STRENGTH (TSF) CLASSIFICATION OO STRENGTH (TSF)	DESCRIPTION		MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%) PLASTICITY	WELL COMPLETION
1	0.0-0.5' Sandy SI loose, massive, o 0.5-14.0' GRAVE Light reddish brov dense, massive, a	iLT: Dark brown, dry to slightly moist rganic debris, scattered gravel [TOPS] and COBBLES with clayey sand myn, slightly moist to moist, loose to my shout 50-60% subangular to angular (a. 30% fine to coarse grained sand, 10 CIAL DEPOSITS]	SOIL] / atrix: edium gravel				



MLBRCHP2.GPJ WOMACK.GDT

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TEST HOLE LOG

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Geotechnical Engineering Geology DATE: 7-9-01 PROJECT NAME: Moonlight Basin Ranch HOLE NO.: TP-01-5 PROJECT LOCATION: Cowboy Heaven, Phase 2 TEST HOLE LOCATION: Phase 2 cabins, west portion, near road switchback MEASURED FROM: TOTAL DEPTH (Ft.): 14 GROUNDWATER LEVEL (Ft.): NA ELEVATION G.S. (Ft.): 8220 DRILLER: Clayton LOGGED BY: gsv DRILL TYPE: Kobelco SK150LC HAMMER: DRILL CO: MBR UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) S.P.T. (N) BLOWS/FT. MOISTURE PLASTICITY DEPTH (Ft.) COMMENTS: NDEX (%) LIQUID LIMIT (%) SAMPLE 14 0.0-0.5' Sandy SILT with gravel: Dark brown, dry to slightly moist, very loose, roots and organic debris [TOPSOIL]
0.5-14.0' GRAVEL and COBBLES with a silty sand matrix: Yellowish to reddish brown, moist, loose to medium dense, crude stratification in 1' to 3' layers, 60-70% angular andesite gravel and cobbles to 10", 20-30% fine to coarse grained D sand, 10-20% non plastic fines [GLACIAL/TALUS] Note: Boulder layer at 10-12', andesite and purple SANDSTONE boulders to 1.5' D.



HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT

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TEST HOLE LOG

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Geotechnical Engineering Geology DATE: 7-9-01 PROJECT NAME: Moonlight Basin Ranch PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-6 TEST HOLE LOCATION: Phase 2 cabins, west end of upper switchback GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: TOTAL DEPTH (Ft.): 10 ELEVATION G.S. (Ft.): 8196 DRILLER: Clayton LOGGED BY: gsv DRILL TYPE: Kobelco SK150LC HAMMER: DRILL CO: MBR UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) PLASTICITY INDEX (%) S.P.T. (N) BLOWS/FT. DEPTH (Ft.) MOISTURE COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-1.0' Sandy SILT: Light brown, slightly moist, loose [TOPSOIL] 1.0-3.5' Clayey GRAVEL: Grayish to reddish brown, moist, medium stiff, massive [COLLUVIUM] 3.5-10.0' Interbedded fine sandstone, siltstone and shale: 1" to 1' layers, hard, medium strong to strong, block fracture with clay infillings, bedding orientation 150° /26° NE [ALBINO, MUDDY and THERMOPOLIS FORMATION UNDIFFERENTIATED]



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TEST_HOLE_LOG2_MLBRCHP2.GPJ_WOMACK.GDT_8/9/01

PRO		VAME	: Moor	ılight E	Basin R	anch					D	ATE:	7-9-01		
PRO	JECT I	-OCA	TION:	Cowbo	y Heav	ven, Ph	nase 2				Н	OLE N	D.: TP	-01-7	
TEST	HOLE	ELOC	ATION:	Ridg	e ~250	' west	of upper switchback								
ELEV	'ATION	l G.S.	(Ft.): 8	3216	тс	OTAL D	EPTH (Ft.): 10	GROUNDWATER LEVEL (Ft.)	: NA	MEAS	SURE	ED FRO	DM:		
DRIL	L TYPE	≣: Ko	belco S	K150L	G HAN	MER:		DRILL CO: MBR	DRII	LLER:	Clay	rton	LOG	GED E	Y: gsv
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION		FOLL	MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1	24 2						0.5-2.0' Bentoniti 2.0' on west end) 2.0-10.0' SHALE moderately strong Note: Appears to orientation becaus	o clayey SILT [TOPSOIL] ic CLAY: (Depth varies from 0' o, yellowish brown, slightly moist, : Grayish to dark brown, moist, g, friable to blocky fracture, thinly be inplace bedrock. Difficult to se rock is highly fractured. Bedo kimately 120-130° / 30° NE [ALBI ERMOPOLIS FORMATION ATED]	weak to bedded get beddir	d,					
6															
111 112 113 113 113 113 114 115 115 115 115 115 115 115 115 115															
8 9				-											



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TEST HOLE LOG

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DATE: 7-9-01 PROJECT NAME: Moonlight Basin Ranch PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-8 TEST HOLE LOCATION: About 450' west of upper switchback GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: TOTAL DEPTH (Ft.): 8 ELEVATION G.S. (Ft.): DRILL TYPE: Kobelco SK150LC HAMMER: DRILLER: Clayton LOGGED BY: gsv DRILL CO: MBR GRAPHICAL LOG UNCONFINED STRENGTH (TSF) CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) PLASTICITY INDEX (%) S.P.T. (N) BLOWS/FT. DEPTH (Ft.) MOISTURE COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-0.5' Sandy SILT: [TOPSOIL] 0.5-2.5' Silty fine SAND: Light brown, slightly moist, loose, massive, scattered platy sandstone gravel [COLLUVIUM] 2.5-8.0' SANDSTONE: Very fine to fine grained, light grayish brown, slightly moist, moderately strong, thinly bedded, closely fractured Note: Intact bedrock, bedding orientation 150° / 33° NE, 148° / 25° NE [ALBINO, MUDDY and THERMOPOLIS FORMATION UNDIFFERENTIATED] 12



TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

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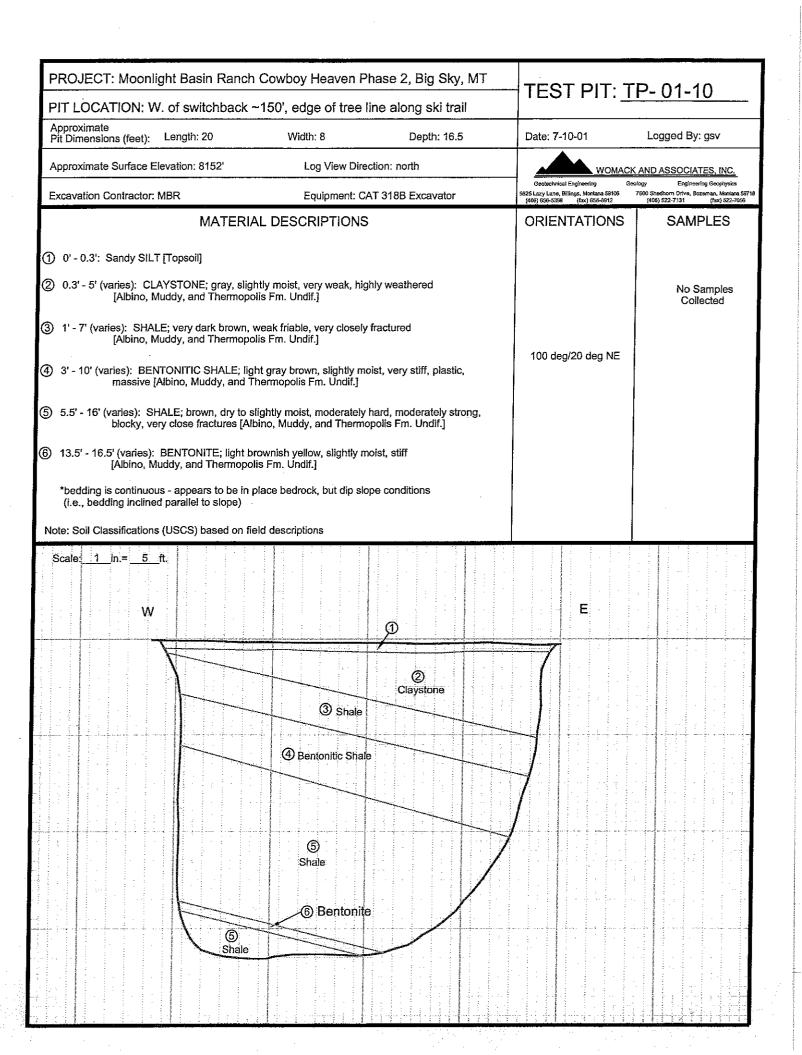
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TEST HOLE LOG

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PROJECT	NAME	: Moon	light E	Basin Ra	anch					D	ATE:	7-9-01		
PROJECT	LOCA	TION: 0	Cowbo	y Heav	en, Pha	ase 2				н	OLE NO).: TP	-01-9	
TEST HO	E LOC	ATION:	Norti	nwest of	f switch	back, west of south	vest corner of Lot #1							
ELEVATIO	N G.S.	(Ft.): 8	132	то	TAL DE	EPTH (Ft.): 17	GROUNDWATER LEVEL (Ft.):	NΑ	ME	ASURE	ED FRO	M:		
DRILL TY	PE: Ko	belco SI	K150L	с нам	MER:		DRILL CO: MBR	DRIL	LER:	Clay	ton	LOG	GED E	SY: gs
DEPTH (Ft.) GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 2 3 3 3 4 4 5 5 5 6 6 7 7 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9						sand matrix: Ligi (pit walls collapsi [LANDSLIDE DE	SHALE and some sandstone with clint brown and gray mixed, moist, verying), randomly oriented rock fragments.	/ loose its						
						very moist, very lo [LANDSLIDE DEE 15.0-17.0' BENTO stiff, (0.75-1.5 tsf)	GRAVEL: Light brownish gray, mo lose to loose, angular shale clasts BRIS] ONITE: Light gray to white, moist, so below 17', [possible lower shear zor mit at 17', very dangerous pit did no	oft to						





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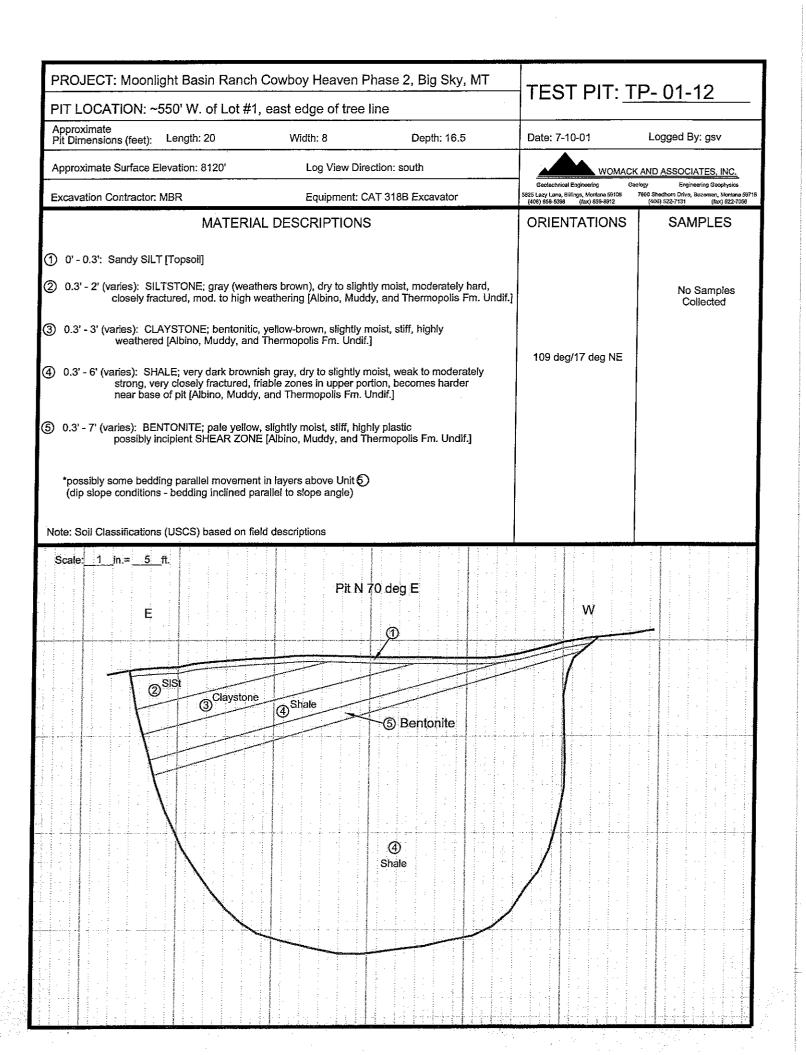
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TEST HOLE LOG

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Geotechnical Engineering Geology

PROJECT	NAME:	: Moonl	ight B	asin Raı	nch					DA	ATE: 7	'-10-01		
PROJECT I	LOCAT	TON: C	owbo	y Heave	n, Pha	se 2				Н	DLE NC).: TP	-01-11	
TEST HOLE	E LOC	ATION:	~300'	west of	Lot #	1								
ELEVATION	N G.S.	(Ft.): 8	122	тот	AL DE	EPTH (Ft.): 16	GROUNDWATER LEVEL (Ft.):	NA	MEA	SURE	D FRO	M:	••	
DRILL TYPI Excavator	E: CA	T 318B		нами	MER:		DRILL CO: MBR	DRIL	LER:	Clay	ton	LOG	GED B	Y: gsv
DEPTH (Ft.) GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 2 3 4 5 6 6 7 8 9 7 7 8 9 7 7 7 8 9 7 7 7 8 9 7 7 7 7	P11-					8.0-9.0' BENTON stiff (1.5 tsf), plast Orientation at bas shear 70°/55° S 9.0-16.0' SHALE: moist, moderately weath prominent 1/4" thi	varies), sandy SILT with scattered (ded SHALE CLAYSTONE: Light to ble, thinly bedded to laminated, his to beds - possibly some disTORTIO 14°NE [ALBINO, MUDDY and FORMATION UNDIFFERENTIATE of the bedden to be bentonite, top of shale 160°/9: Very dark gray brown, dry to slig to strong, thinly bedded, blocky fractive plantick fractures 70° / 55° S of and THERMOPOLIS FORMATION (TED)	moist, moist, NE, httly ture, ies,						





TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

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TEST HOLE LOG

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DATE: 7-10-01

Geotechnical Engineering Geology

PROJECT NAME: Moonlight Basin Ranch PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-13 TEST HOLE LOCATION: ~550 northwest of northwest corner Lot #1, east edge tree line ELEVATION G.S. (Ft.): 8076 TOTAL DEPTH (Ft.): 16 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: DRILL TYPE: CAT 318B HAMMER: DRILL CO: MBR DRILLER: Clayton LOGGED BY: gsv Excavator UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) PLASTICITY INDEX (%) DEPTH (Ft.) S.P.T. (N) BLOWS/FT. MOISTURE COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-7.0' GRAVELLY CLAY: Light yellowish brown to gray, slightly moist to wet at minor seep zones, medium stiff, structureless (randomly oriented, discontinuous shale layers [LANDSLIDE DEBRIS] 7.0-8.0' BENTONITE: Pale yellow to white, slightly moist, stiff to very stiff (1.5 tsf), highly plastic, scattered angular shale gravel and cobbles, orientation 129°/18° NE at base of bentonite/top of shale [SHEAR ZONE]
8.0-16.0' SHALE: Very dark grayish brown, dry to slightly moist, moderately strong, thinly bedded to laminated, blocky fracture, minor small seeps [ALBINO, MUDDY and THERMOPOLIS FORMATION UNDIFFERENTIATED] Note: Hard, difficult digging near base of pit



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TEST HOLE LOG

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PROJEC	CT N	AME:	: Moon	light B	asin R	anch	·		<u> </u>		D	ATE:	7-10-01		
PROJEC	CT LC	DCAT	TON: (Cowbo	y Heav	ren, Ph	nase 2				Н	OLE NO	D.: TP	-01-14	
TEST H	IOLE	LOCA	ATION:	Abou	t 250' r	orthw	est of northwest corne	er Lot #1							
ELEVAT	TION :	G.S.	(Ft.): 8	3164	то	TAL C	EPTH (Ft.): 17	GROUNDWATER LEVEL (Ft.): NA	ME	ASURE	D FRO	OM:		
DRILL T		CA	T 318B		НАМ	MER:		DRILL CO: MBR	DR	ILLER:	Clay	ton	LOG	GED E	SY: gsv
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1—2—2—2—2—2—2—2—2—2—2—2—2—2—2—2—2—2—2—2							Light yellowish bu	L and COBBLES in a bentonit rown to gray, slightly moist to lium stiff, structureless (rando ale layers [LANDSLIDE DEBF	wet at minor mly oriented	x:					
5 6 7 8 9 9							5.0-10.0' SHALE fracture, thickness shearing at lower shale [LANDSLID	: Very dark gray to black, dry s varies from 1 - 3', some slic contact, orientation 140º/20º i E DEBRIS]	/, hard, block kensides and NE at base o	y d if					
10 11 12 13 14 15 16 16 16 16 16 16 16 16 16 16 16 16 16	TATES A STAN STAN STAN STAN STAN STAN STAN S						weak to moderate blocky fracture (ap	E: Medium brown, dry to sligh ly strong, thinly bedded to lan opears to be in place bedrock RMOPOLIS FORMATION TED]	niлated,						
18-119-119-119-119-119-119-119-119-119-1								· · · · · · · · · · · · · · · · · · ·							



TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT

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TEST HOLE LOG

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PROJECT NAME: Moonlight Basin Ranch

DATE: 7-10-01

PROJECT LOCATION: Cowboy Heaven, Phase 2

HOLE NO.: TP-01-15

TEST HOLE LOCATION: ~100' north/northwest of the northwest corner of Lot #1 TOTAL DEPTH (Ft.): 14 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: ELEVATION G.S. (Ft.): 8044 DRILLER: Clayton LOGGED BY: gsv DRILL TYPE: CAT 318B HAMMER: DRILL CO: MBR Excavator GRAPHICAL LOG UNCONFINED STRENGTH (TSF) CLASSIFICATION DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) RECOVERY PLASTICITY INDEX (%) MOISTURE DEPTH (Ft.) S.P.T. (N) BLOWS/FT. COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-1.0' Sandy SILT with scattered gravel: Light to dark 7 N. 1 brown, slightly moist, very loose, roots and organic debris ITOPSOILI 1.0-12.0' GRAVEL and COBBLES with a CLAY matrix: Color mixed, light to dark brown, moist, loose, discontinuous lenses and pockets of shale [LANDSLIDE DEBRIS] 12.0-13.5' BENTONITE: White to pale yellow, moist to wet, soft to medium stiff (0.25 to 0.75 tsf), minor seepage along bottom contact, appears to dip about 10-15° NE [SHEAR ZONE] 13.5-14.0' SHALE: Black, dry to slightly moist, moderately strong, difficult digging [ALBINO, MUDDY and THERMOPOLIS FORMATION UNDIFFERENTIATED] 15 Note: Excavator refusal at 14', very hard. Pit walls unstable, did not enter pit below 9' 18



HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

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TEST HOLE LOG

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Geotechnical Engineering DATE: 7-10-01 PROJECT NAME: Moonlight Basin Ranch HOLE NO .: TP-01-16 PROJECT LOCATION: Cowboy Heaven, Phase 2 TEST HOLE LOCATION: ~150' north of Lot #3 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: TOTAL DEPTH (Ft.): 20 ELEVATION G.S. (Ft.): 8032 LOGGED BY: gsv HAMMER: DRILL CO: MBR DRILLER: Clayton DRILL TYPE: CAT 318B Excavator UNCONFINED STRENGTH (TSF) CLASSIFICATION GRAPHICAL LO DRY DENSITY (PCF) RECOVERY (%) WELL COMPLETION DESCRIPTION CONTENT (%) PLASTICITY INDEX (%) MOISTURE DEPTH (Ft.) S.P.T. (N) BLOWS/FT. COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-0.5' Sandy SILT: Very dark brown, slightly moist, very loose, roots and organics [TOPSOIL] 0.5-15.0' GRAVEL and COBBLES with a clayey sand matrix: Reddish brown, slightly moist to moist (becomes very moist at 8'), loose to medium dense, massive, angular shale gravel and small cobbles, about 60% angular to subangular andesite gravel and cobbles to 10", 20-30% fine to coarse grained sand, 10-20% plastic fines [GLACIAL DEPOSITS/LANDSLIDE] 15.0-20.0' GRAVEL and COBBLES with a sandy clay matrix: Brown to light brown, very moist to wet at ~19' (seepage into pit), loose, structureless, subangular to angular shale cobbles to 6", at 17' SHALE cobbles in clayey matrix Note: Pit collapsing at 17', walls collapsing - did not enter pit



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TEST HOLE LOG

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Geolechnical Engineering Geology

PROJECT NAM	ME: Moon	light B	asin Ra	nch					DA	ATE: 7	'-10-01		
PROJECT LOG	CATION: (Cowbo	y Heave	en, Pha	ase 2				Н	DLE NO).: TP	-01-17	
TEST HOLE LO	OCATION:	Abou	t 200' n	orthwe	est of pond, ~300' wes	est of northwest corner Lot 13							
ELEVATION G	.S. (Ft.): 7	964	то	TAL D	EPTH (Ft.): 17	GROUNDWATER LEVEL (Ft.):	NA	MEA	ASURE	D FRC	M:		
DRILL TYPE: Excavator	CAT 318B		HAM	MER:		DRILL CO: MBR	DRII	LER:	Clay	ton	LOG	GED B	Y: gsv
Ft.) SAL LOG	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1 2 3 4 5 5 6 6 7 7 8 8 9 10 11 11 12 13 14 15 15 16 17 17 17 17 17 17 17 17 17 17 17 17 17					sandy CLAY mater black, slightly mo angular shale and DEPOSITS] 7.0-9.0' BENTON 9.0-11.0' GRAVE sandy CLAY mater black, slightly moi angular shale and DEPOSITS] 11.0-11.5' BENTON 11.5-17.0' GRAVE sandy CLAY mater black. slightly moi sangular shale and DEPOSITS]	EL, COBBLES and small BOULDER trix: Color mixed, light to dark brow pist to very moist, very loose to loos d scattered andesite clasts [LANDS NITE: Discontinuous layer EL, COBBLES and small BOULDE rix: Color mixed, light to dark brow ist to very moist, very loose to loos d scattered andesite clasts [LANDS ONITE: /EL, COBBLES and small BOULDE rix: Color mixed, light to dark brow ist to very moist (wet at 15'), very lo ale and scattered andesite clasts POSITS]	RS with n, gray te, SLIDE	0					
9													



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TEST HOLE LOG

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Geotechnical Engineering PROJECT NAME: Moonlight Basin Ranch DATE: 7-10-01 PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO .: TP-01-18 TEST HOLE LOCATION: West of proposed road switchback, ~100' west of Lot #21 ELEVATION G.S. (Ft.): 7916 TOTAL DEPTH (Ft.): 15 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: DRILL TYPE: CAT 318B HAMMER: DRILLER: Clayton LOGGED BY: gsv DRILL CO: MBR UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION DRY DENSITY (PCF) RECOVERY (%) MOISTURE CONTENT (%) DESCRIPTION WELL COMPLETION S.P.T. (N) BLOWS/FT. DEPTH (Ft.) PLASTICITY COMMENTS: NDEX (% LIMIT (%) SAMPLE LIQUID 0.0-1.5' Silty CLAY: Dark brown, slightly moist to moist, soft, massive, organics [TOPSOIL] 1.5-15.0' Mixed silty CLAY and SHALE: Very dark grayish brown, moist to wet (seepage at 8', pit filling with water at 12'), medium stiff clay, weak, friable shale, random orientation, becomes harder and more shale at 14' - difficult to see below water [LANDSLIDE DEPOSITS] Note: Pit walls collapsing - did not enter pit below 8' 15



HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

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TEST HOLE LOG

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Geotechnical Engineering Geology PROJECT NAME: Moonlight Basin Ranch DATE: 7-11-01 PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-19 TEST HOLE LOCATION: South of Lot #2, south side of road ELEVATION G.S. (Ft.): 8124 TOTAL DEPTH (Ft.): 15 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: DRILLER: Clayton DRILL TYPE: CAT 318B HAMMER: DRILL CO: MBR LOGGED BY: gsv Excavator UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION CONTENT (%) S.P.T. (N) BLOWS/FT. DEPTH (Ft.) PLASTICITY MOISTURE COMMENTS: LIQUID LIMIT (%) INDEX (%) SAMPLE 24 N. 7 0.0-1.5' Sandy SILT: Medium to dark brown, dry to slightly moist, loose, massive, roots and organic debris [TOPSOIL] 1.5-15.0' Sandy GRAVEL and COBBLES: Some clayey layers, light brown to reddish brown, dry to slightly moist (water seeping into pit at 14'), becomes moist at 10', medium dense, crude stratification in 1'-3' layers (parallel to ground slope), about 60% angular to subangular shale, sandstone and siltstone gravel and cobbles to 10", 25-30% fine to coarse grained sand, 10-15% fines [COLLUVIUM] Note: Harder digging at 15', increase in shale cobble probably top of shale bedrock



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TEST HOLE LOG

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PRO	JECT L	OCAT	TON: C	owbo	y Heave	∍n, Pha	ise 2					Н	OLE NO).: TP	-01-20	-
TEST	HOLE	LOC	ATION:	South	neast co	orner of	Lot #4, north side o	f road								
ELEV	'ATION	G.S.	(Ft.): 8	108	то	TAL DE	EPTH (Ft.): 15	GROUNDWA	TER LEVEL (Ft.):	AV	MEA	SURE	D FRO	M:		
DRILL TYPE: CAT 318B HAMMER								DRILL CO: N	İBR	DRIL	LER:	Clay	ton	LOGGED BY:		
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRI	PTION			MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL
1 2 3 4 5 6 7 8 9 10 11 12 13 13 14 15 5 6 6 11							sand matrix: Re- medium dense, r andesite and sha sand, 20% plasti	ddish brown, slignassive, about of the clasts to 8", 2 clasts	S with sandy clay to hitly moist to very moist to very moio% angular to subation fine to coarse gold moio fine fine fine fine fine fine fine fine	ioist, ngular rained						
7 8 9			-													



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TEST HOLE LOG

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Geotechnical Engineering Geology

TEST_HOLE_LOG2_MLBRCHP2.GPJ_WOMACK.GDT_8/9/01

							·								
PRO	TOBL	VAME	: Moon	light B	asin Ra	anch	·····		-		D/	ATE: 7	⁷ -11-01		
PRO	JECT L	_OCA	TION: C	Cowbo	y Heave	en, Pha	ase 2				н	OLE NO).: TP	-01-21	
TEST	HOLE	LOC	ATION:	South	side o	f Lot #7	7, north side of road								
ELEV	ATION	1 G.S.	(Ft.): 8	104	то	TAL DE	EPTH (Ft.): 18	GROUNDWATER LEVEL (Ft.):	NA	MEAS	SURE	D FRC	M:		
DRILI Excav		Ξ: C,	AT 318B		НАМІ	MER:		DRILL CO: MBR	DRI	LER:	Clay	ton	LOG	GED E	Y: gsv
DЕРТН (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION		TOICH	MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL COMPLETION
1							Reddish brown, n subangular to and and shale), 25-30 plastic fines, no s DEPOISTS/COLL	EL and COBBLES with a clayey s noist, loose, massive, about 60-7 jular gravel and cobbles to 10" (a % fine to coarse grained sand, 5- eepage [GLACIAL .UVIUM] opsoil stripped by dozer	70% Indesite						
19-							•								



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TEST HOLE LOG

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Geotechnical Engineering Geology

TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

PRO.	JECT N	NAME	: Moon	light B	asin R	anch				D	ATE:	7-11-01		
PRO	JECT L	.OCAT	FION: (Cowbo	y Heav	/en, Ph	nase 2			Н	OLE N	D.: TP	-01-22	
TEST	r HOLE	LOC	ATION:	End o	of cul-c	ie-sac,	lower road, northeast	t of Lot 23						
ELEV	/ATION	ı G.S.	(Ft.): 7	7856	ТС	OTAL D	PEPTH (Ft.): 18	GROUNDWATER LEVEL (Ft.): N	Α	MEASUR	ED FRO	DM:		-
DRILI Excav		: CA	T 318B		HAM	MER:		DRILL CO: MBR	DRIL	LER: Clay	/ton	LOG	GED E	Y: gsv
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION		MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)		WELL COMPLETION
1		22-10					sandy clay matrix mostly massive w about 60% angula andesite and shal above clay [LAND 9.0-15.0' Gravelly to very stiff, massi coarse grained sa cobbles [LANDSL 15.0-18.0' GRAVE matrix: Light brow massive, andesite angular gravel and	and COBBLES with a clayey sand to Reddish brown, loose to medium or ith occasional thin lenses of sandy gar to subrounded gravel and cobbles to clasts, 40% matrix, seepage at 7-DSLIDE DEPOSITS] TO CLAY: Gray, slightly moist to moist to about 50-60% plastic clay, 40-50 and, subrounded gravel and scattered.	dense, gravel, ,, 8'					
9=														



TEST_HOLE_LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

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TEST HOLE LOG

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			: Moon								DATE:			
PROJE	CT L	OCAT	TION: (Cowbo	y Heave	en, Ph	ase 2			i	HOLE N	O.: TP	-01-23	
TEST H	HOLE	LOC	ATION:	North	side of	f Lot #:	24	Γ						
ELEVA.	MOIT	G.S.	(Ft.): 7	845	ТО	TAL D	EPTH (Ft.): 17	GROUNDWATER LEVEL (Ft.):	: NA	MEASUR	RED FRO	OM:		
DRILL T Excavat		: CA	T 318B	1	HAMi	MER:		DRILL CO: MBR	DRIL	LER: Cla	yton	LOG	GED E	SY: gsv
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTION		MOISTURE	DRY DENSITY (PCF)	LIQUID LIMIT (%)		WELL COMPLETION
= 2	71.17						0.0-1.0' Sandy S	ILT: [TOPSOIL]						
2 3 4 5 7 8 9 8 10 11 11 2 3 4 4 7							Light brown, mois angular shale and 25-30% fine to co increase in large	EL and COBBLES with clayey sait, loose, massive, about 60% sulf sandstone gravel and cobbles tarse grained sand, 10-15% plast cobbles and small boulders with a unstable [COLLUVIUM/LANDSI	bangular to o 10", ic clay, depth, no	0				
8														



HOLE_LOG2 MLBRCHP2.GP3 WOMACK.GDT

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TEST HOLE LOG

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Geology PROJECT NAME: Moonlight Basin Ranch DATE: 7-11-01 PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-24 TEST HOLE LOCATION: South side of Lot #34 ELEVATION G.S. (Ft.): 7872 TOTAL DEPTH (Ft.): 17 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: LOGGED BY: gsv DRILLER: Clayton DRILL TYPE: CAT 3188 HAMMER: DRILL CO: MBR Excavator UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION WELL COMPLETION CONTENT (%) PLASTICITY INDEX (%) DEPTH (Ft.) S.P.T. (N) BLOWS/FT. MOISTURE COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-1.0' Sandy SILT: [TOPSOIL] 1.0-1.8' Gravelly silty SAND: Light brown, slightly moist, loose, massive [COLLUVIUM]
1.8-17.0' SHALE: Dark brown to dark gray, dry to slightly moist, thinly bedded to laminated, weak, friable, highly weathered in upper 1.5' (residual soil), moderately weathered below 3', becomes hard at 17' [ALBINO, MUDDY and THERMOPOLIS FORMATION UNDIFFERENTIATED] Bedding orientation 110°/14° NE 12



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TEST HOLE LOG

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Geotechnical Engineering Geology

LOG2 MLBRCHP2.GPJ WOMACK.GDT 8/9/01

PROJECT NAME: Moonlight Basin Ranch DATE: 7-11-01 HOLE NO.: TP-01-25 PROJECT LOCATION: Cowboy Heaven, Phase 2 TEST HOLE LOCATION: North side of Lot #25 ELEVATION G.S. (Ft.): 7860 TOTAL DEPTH (Ft.): 20 GROUNDWATER LEVEL (Ft.): NA MEASURED FROM: DRILL TYPE: CAT 318B HAMMER: DRILLER: Clayton LOGGED BY: gsv DRILL CO: MBR Excavator UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) WELL COMPLETION DESCRIPTION MOISTURE CONTENT (%) PLASTICITY INDEX (%) DEPTH (Ft.) S.P.T. (N) BLOWS/FT. COMMENTS: LIQUID LIMIT (%) SAMPLE 0.0-1.0' Sandy SILT: Dark brown, slightly moist, loose, organics [TOPSOIL] 1.0-3.0' Gravelly silty SAND: Light brown, slightly moist, loose, massive [COLLUVIUM] 3.0-6.0' Sandy GRAVEL: Very dark grayish brown, dry to slightly moist, medium dense, massive, entirely shale clasts 1" to 3", angular, random orientation [COLLUVIUM] 6.0-20.0' SHALE: Dark brown to dark gray, dry to slightly moist, thinly bedded to laminated, weak, friable, highly weathered in upper 1.5' (residual soil), moderately weathered below 3', very closely fractured, some interbedded siltstone [ALBINO, MUDDY and THERMOPOLIS FORMATION **UNDIFFERENTIATED** 10



TEST_HOLE_LOG2_MLBRCHP2.GPJ_WOMACK.GDT_8/9/01

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TEST HOLE LOG

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PROJECT NAME: Moonlight Basin Ranch

DATE: 7-11-01

PROJECT LOCATION: Cowboy Heaven, Phase 2 HOLE NO.: TP-01-26

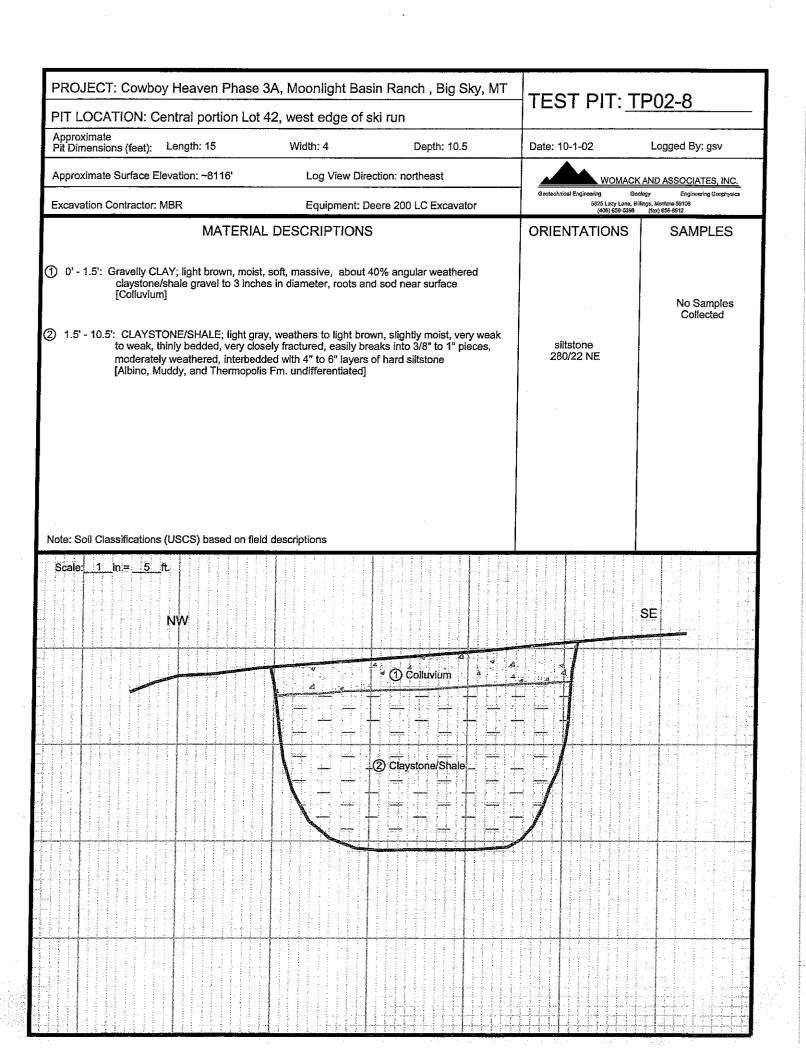
TEST	HOLE	E LOC	ATION:	South	n side o	f Lot #3	36						-		
ELEV	OITA	I G.S.	(Ft.): 7	904	тот	TAL DE	EPTH (Ft.): 17	GROUNDWATER I	_EVEL (Ft.): N	A N	/EASUR	ED FRO	M:		
DRILI Excav		E: CA	T 318B		НАМ	MER:		DRILL CO: MBR		DRILLE	ER: Clay	ton	LOC	GED E	Y: gsı
DEPTH (Ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS:	DESCRIPTIO	DN		MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	WELL
	1 7 1. 7 1. 7.						0.0-1.0' Sandy Si	LT: [TOPSOIL]		•					
2 3 3							brown, moist, loos sandstone gravel	RAVEL with COBBL ie, massive, predomi and cobbles in a san nes at lower contact	nantly siltstone dy clay matrix, s	and			İ		
5 6							moist, weak, friabl fractured, orientati	Very dark grayish b e, thinly bedded to la on 110 ⁹ /20 ⁹ NE [ALI ORMATION UNDIFI	minated, very c SINO, MUDDY a	losely and					
3															
)												-			
						-			٠.				-		, -
									•						
										•					

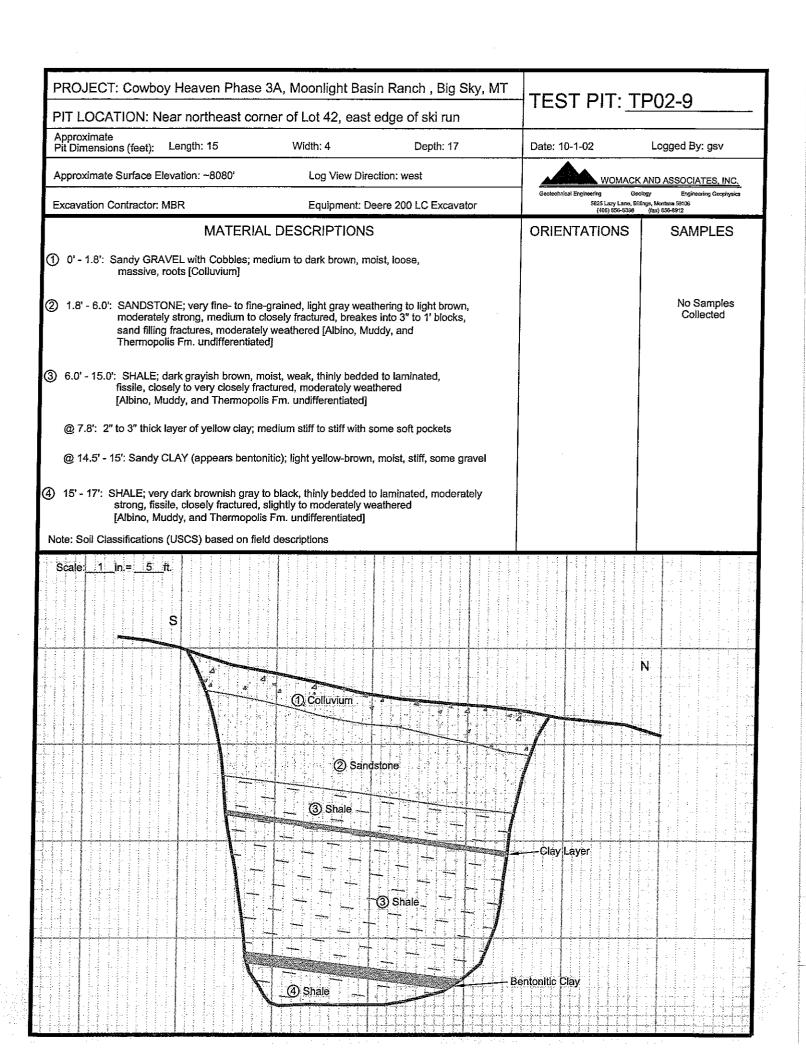
DE	PO IECT: Cowb	ov Heaven Phas	o 2 Magniight Pagin	Ranch , Big Sky, MT			
				about 15' north of road	TEST P	IT: <u>T</u>	P02-4
Apı	proximate Dimensions (feet):	Length: 15	Width: 8	Depth: 13	Date: 7-10-02		Logged By: gsv
App	proximate Surface E	Elevation: 7956'	Log View Direc	ction: south			(AND ASSOCIATES, INC.
Exc	cavation Contractor:		Equipment: CA	AT Excavator	Geotechnical Engineerin		ology Engineering Geophysics Illings, Montana 59106 (fax) 656-8912
		MATER!	AL DESCRIPTIONS	•	ORIENTAT	ONS	SAMPLES
① (massive,	about 30-40% subar	ish brown, slightly moist, longular gravel to 3" in diame low plastic fines [Colluviur	eter. 40-50% fine- to			No Samples Collected
2 6	6' - 11': Sandy GRA cobbles t	NEL; as above excep o 6" in diameter [Coll	ot about 50-60% subanguli uvium]	ar gravel, scattered		İ	33.1361.04
③ 1	banding,	about 30% fine-grain	ight gray to pale yellow, m ed sand, 15% subangular opears bentonitic) [reworke	oist, stiff, some oxidation siltstone gravel to 3" in ed bentonite layer/Colluvium]			
④ 1	3' - 13.5': SILTSTC	DNE; light grayish bro	wn, hard [Albino, Muddy, a	and Thermopolis Fm. undiff.]			
Note	o Call Olanaisantian	(UCOO) based on F				į	
1 1 1	ale: 1 in= 5	s (USCS) based on fi	eia aescriptions				
3	ae						W (road)
			Colluvien			tender (1985) and the tender (1985) and the	
			a Ž Colluviu		Control of the contro		
7			3 Bentoniti	c Clay			
				♣ Silfistorie			

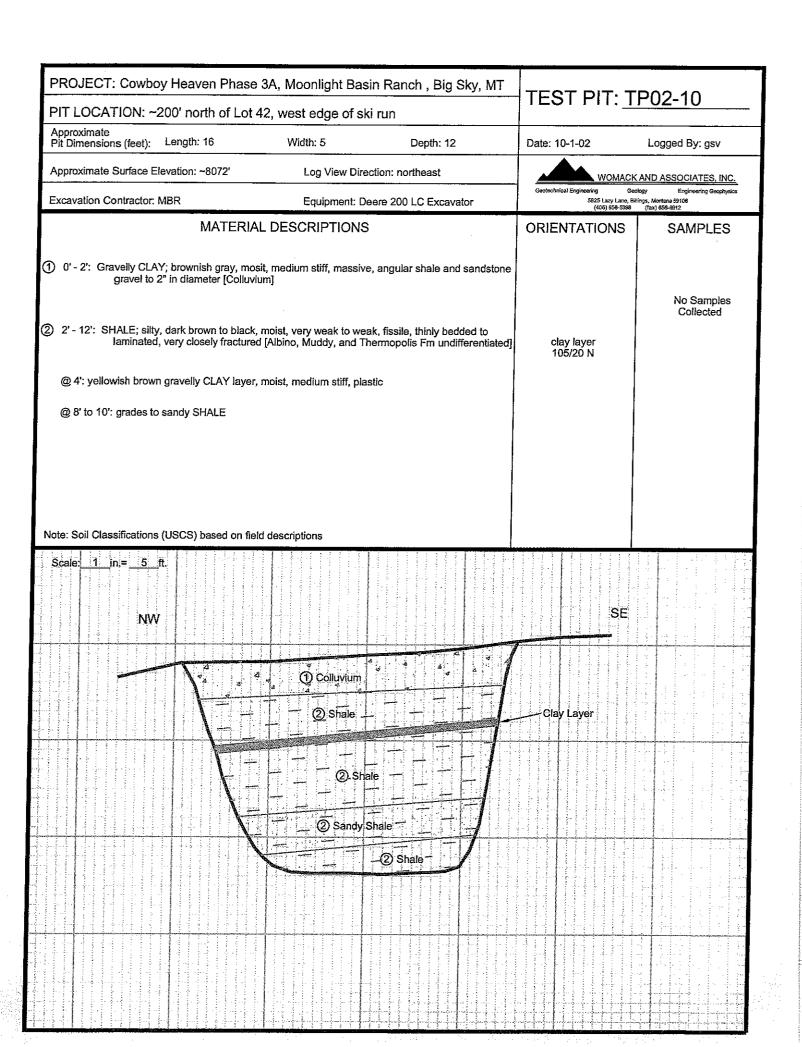
PROJECT: Cowboy Heaven Pha	se 3, Moonlight Basin	Ranch , Big Sky, MT	TEST PIT:]	TP02-5
PIT LOCATION: about 100' north	of SW corner Lot 20		ILOITII.	11 02-0
Approximate Pit Dimensions (feet): Length: 17	Width: 12	Depth: 18	Date: 7-10-02	Logged By: gsv
Approximate Surface Elevation: ~7940'	Log View Direc	tion: south		K AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: CA	T Excavator		eology Engineering Geophysics Billings, Montana 59106 9 (fax) 856-8912
MATER	RIAL DESCRIPTIONS		ORIENTATIONS	SAMPLES
0' - 11.5' west end, 0' - 14.5' east end: about 40-60% angular, platy clay matrix, dark brown, stiff 0.5' thick: Sandy CLAY with scattered very moist, stiff, oxidation ba shale contact [reworked ben	shale and siltstone clasts, a , crude stratification parallel angular gravel; light yellowisi inding, appears to be benton	bout 40-60% sandy plastic to slope [Colluvium]		No Samples Collected
3 12' - 18' west end, 15' - 18' east end: SI weak, friable [Albino, Muddy Note: Soil Classifications (USCS) based on	, and Thermopolis Fm. undifi	inly bedded to laminated,		
E -	d Collu	yum		W
	Bentonitic Clay			
	seepage	3 Shale		

PROJECT: Cowboy Heaven Phase	e 3, Moonlight Basin Ranch	, Big Sky, MT	TEST DIT: T	
PIT LOCATION: Lot 12, centerline	ski run right-of-way		TEST PIT: T	- UZ-U
Approximate Pit Dimensions (feet): Length: 12	Width: 10	Depth: 16	Date: 7-11-02	Logged By: gsv
Approximate Surface Elevation: ~8020'	Log View Direction: sou	oth		K AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: CAT Excav	ator		eology Engineering Geophysics Billings, Montana 59106 (fax) 556-8912
MATERI	AL DESCRIPTIONS		ORIENTATIONS	SAMPLES
stratified parallel to slope, abo sandstone, and andesite, abo [Colluvium/possibly older land: (2) 12' - 16': SANDSTONE; light brownish gr	at 11' (seepage), loose to medium ut 60% subangular to angular clast ut 40% sandy plastic clay matrix (p slide deposits]	dense, crudely is of siltstone, it walls collapsing)		No Samples Collected
[Albino, Muddy, and Thermopo	olis Formation undifferentiated]	·		
,				
Note: Soil Classifications (USCS) based on fi	eld descriptions			
Scale: 1 in ≡ 5 ft.				W
	(i) Colluvium			
	seepage 2 Sandstone			

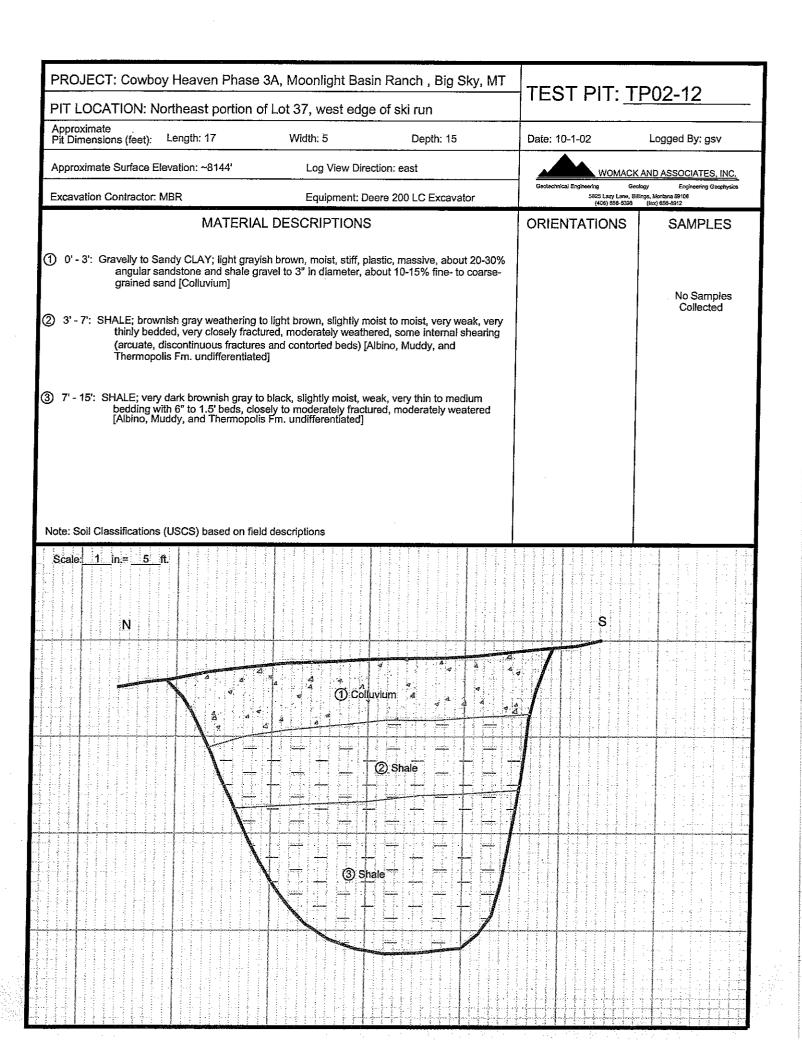
PROJECT: Cowboy Heaven Phase	e 4 Moonlight Rasin	Ranch Big Sky MT		<u>.</u>
PIT LOCATION: east side Pony Ex	-		TEST PIT: I	P02-7
Approximate Pit Dimensions (feet): Length: 15	Width: 8	Depth: 16	Date: 7-11-02	Logged By: gsv
Approximate Surface Elevation: ~7760'	Log View Direc		WOMAC	CAND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: CA	AT Excavator	Geotechnical Engineering Ge	eology Engineering Geophysics
MATERI.	AL DESCRIPTIONS		ORIENTATIONS	SAMPLES
① 0' - 5': GRAVEL and COBBLES with clay moist, medium dense, stratifie decaying organics [Fill]	vey sand matrix; mixed dat d in 1-foot thick lifts, scatte	rk gray and brown, slightly ered roots, strong odor from		No Samples Collected
2 5' - 6': Sandy SILT; dark reddish brown, s some roots [Buried Topsoil La	slightly moist, soft to medic ayer]	um stiff, massive		
3 6' - 16': GRAVEL, COBBLES, and BOUL slightly moist to moist, medium subangular clasts up to 1.5' in about 30% clayey sand matrix	n dense, massive, about 70 diameter including andesi	0% coarse, angular to te, sandstone,		
Note: Soil Classifications (USCS) based on fi	eld descriptions			
Scale 1 in = 5 ft				
	① Fill			
	3) Colluvium/Glacial Depo	sits		



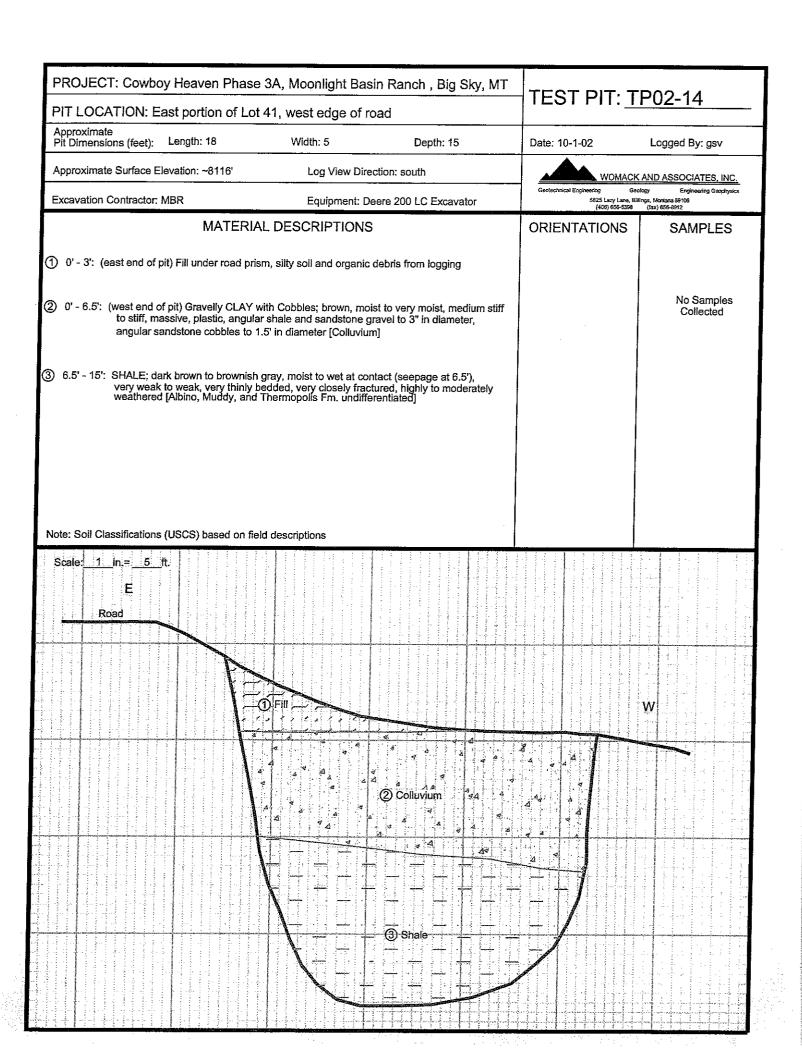




PROJECT: Cowboy Heaven Phase	3A, Moonlight Bas	in Ranch , Big Sky, M	TEST D	T . T	D02 11
PIT LOCATION: Approximately 400'	north of Lot 42, ea	ast edge of ski run	TEST P	i. <u>l</u>	<u> </u>
Approximate Pit Dimensions (feet): Length: 17	Width: 5	Depth: 12	Date: 10-1-02		Logged By: gsv
Approximate Surface Elevation: ~8032'	Log View Dire	ction: west		NOMACK	AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: De	eere 200 LC Excavator	Geotechnical Engineering 582: (4		ology Engineering Geophysics Ilings, Montana 59106 (fax) 656-8912
MATERIA	L DESCRIPTIONS		ORIENTATI	SNC	SAMPLES
0' - 2.4': Sandy CLAY; grayish brown, mois sand and gravel to 1" in diameter	st, medium stiff, massive r (shale gravel), roots [4	e, angular coarse-grained Colluvium]			
2) 2.4' - 8.6': SHALE; light to dark brownish g highly to moderately weathered, shearing (arcuate, discontinuous Thermopolis Fm. undifferentiated	very closely to closely to 1/4" thick clay seam) [/	ractured, some internal			No Samples Collected
@ 5': 3" thick layer of yellow bentonitic CL	AY; slightly moist, stiff,	inclined at ~20 degrees N			
@ 8': 7" thick layer of light yellowish gray b	entonitic CLAY, moist,	medium stiff to stiff			
3 15' - 17': SHALE; very dark brownish gray strong, fissile, closely fractured, [Albino, Muddy, and Thermopol	slightly to moderately v	to laminated, moderately veathered			
Note: Soil Classifications (USCS) based on field	l descriptions				
Scale: 1 In = 5 ft.	descriptions				
S					
					N
	a (1) Coli	ıvium A A A A			
		② Shale			
	② Shale	1	Clay Layer		
	③ Shale	Bent	onitic Clay		



PROJECT: Cowboy Heaven Phase	3A. Moonlight Basir	Ranch Big Sky MT		
PIT LOCATION: East portion of Lot			TEST PIT: 1	P02-13
Approximate Pit Dimensions (feet): Length: 15	Width: 5	Depth: 15	Date: 10-1-02	Logged By: gsv
Approximate Surface Elevation: ~8172'	Log View Direct	tion: east	WOMAC	K AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: Dee	ere 200 LC Excavator		eology Engineering Geophysics illings, Montana 59106 (fax) 656-6912
MATERIA ① 0' - 3': Gravelly to Sandy CLAY; light grayi shale and sandstone gravel to 2	L DESCRIPTIONS sh brown, moist, stiff, pla " in diameter, roots [Collu	istic, massive, angular ivium]	ORIENTATIONS	SAMPLES
② 3' - 12': SHALE; silty, light brownish gray, shighly to moderately weathered, discontinuous fracture planes, re Thermopolis Fm. undifferentiated @ 12': 3" thick layer of light yellowish gray	very closely to closely fra eddish brown mottling [Alt d]	actured, some arcuate, pino, Muddy, and	Shale bed 280/33 N *possibly fracture plane, slightly steeper than bedding (?)	No Samples Collected
3 12.3' - 15': SHALE; very dark brownish gray strong, fissile, closely fractured, [Albino, Muddy, and Thermopolis	slightly to moderately wea	ded to laminated, moderately athered		
Note: Soil Classifications (USCS) based on field	descriptions			
Scale: 1 in = 5 ft.			8,1	
	(1) Colluy	ium •		
	② Shale			
		③ Shale	Bentonitic Clay	



PROJECT: Cowboy Heaven Phase PIT LOCATION: Southwest portion			TEST PI	Γ: <u>Τ</u>	P02-15
Approximate Pit Dimensions (feet): Length: 18	Width: 5	Depth: 18	Date: 10-1-02		Logged By: gsv
Approximate Surface Elevation: ~8116'	Log View Direc	tion: north	w w	OMACK	AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: Dec	ere 200 LC Excavator	Geotechnical Engineering 5825 L (408	Geol azy Lane, Bill 658-5398	ogy Engineering Geophysics ings, Montana 59106 (fax) 656-8912
MATERIA	L DESCRIPTIONS		ORIENTATIO	NS	SAMPLES
0' - 2.5': GRAVEL and COBBLES with Silt massive, subangular sandstone to 5" in diameter [Colluvium] 2.5' - 4.7': Silty SAND; strong reddish brow stratified, scattered highly weath [Glacial Deposits]	and andesite gravel to 3	" in diameter, cobbles massive to faintly			No Samples Collected
4.7' - 18': GRAVEL and COBBLES with cla reddish brown, moist to very moi subangular (some subrounded) (sandstone, siltstone, and andesi and low plastic fines [Glacial Dep	st, medium dense, massi gravel and cobbles to 7" i te), about 40% fine- to co	ive, about 60% n diameter (includes			
ote: Soil Classifications (USCS) based on field	d descriptions			V	
E	⊕ Gradu Sility S	vel and Cebbles		W	
		avel and Cobbles () C			

PROJECT: Cowboy Heaven Phase	3A, Moonlight Bas	in Ranch , Big Skv, MT		
PIT LOCATION: West portion of Lot			TEST PIT: 1	P02-16
Approximate Pit Dimensions (feet): Length: 18	Width; 5	Depth: 17	Date: 10-1-02	Logged By: gsv
Approximate Surface Elevation: ~8154'	Log View Dire	ction: west		K AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: De	eere 200 LC Excavator		eology Engineering Geophysics Illings, Montana 59106 (fax) 656-6912
MATERIA	L DESCRIPTIONS		ORIENTATIONS	SAMPLES
0' - 4': Gravelly Silty SAND; reddish brown parallel to slope, angular to sub in diameter [Colluvium]	, slightly moist, loose, si angular andesite, sands	tratified in 4" to 6" layers tone, and shale gravel to 3"		No Samples
4' - 6.5': Silty SAND with scattered gravel; slightly moist, medium dense, rr	fine- to medium-grained lassive [Glacial Deposits	i, strong reddish brown, s]		Collected
3 6.5' - 14': GRAVEL and COBBLES with silt slightly moist, medium dense, m shale, and andesite gravel and o matrix [Glacial Deposits]	assive, about 70% suba	ngular sandstone, sittstone		
14' - 15.5': Sandy Clayey GRAVEL to Grav dense, massive, subrounded an	elly CLAY; very dark bro desite gravel to 1 1/2" in	own, moist, very stiff and diameter		
15.5' - 17': GRAVEL and COBBLES with si	ify to clayey sand matrix	;; [Glacial Deposits]	ļ	•
Note: Soil Classifications (USCS) based on fiel	d descriptions			
Scale: 1 in.≡ 5 it.				
) Gravelly Silty Sand		
	② Silt	y Sand		
		avel and Cobbles		
	4) Gravell	C0000000000000000000000000000000000000		

PROJECT: Cowboy Heaven Phase	e 3A. Moonlight Bas	sin Ranch . Big Sky MT		
PIT LOCATION: West portion of Lo			TEST PIT: T	P02-17
Approximate Pit Dimensions (feet): Length: 15	Width: 5	Depth: 12	Date: 10-1-02	Logged By: gsv
Approximate Surface Elevation: ~8212'	Log View Dire	ection: west	WOMAC	K AND ASSOCIATES, INC.
Excavation Contractor: MBR	Equipment: D	eere 200 LC Excavator	•	eology Engineering Geophysics illings, Montana 59106 (fax) 856-8912
MATERL	AL DESCRIPTIONS	3	ORIENTATIONS	SAMPLES
O' - 2': Sandy GRAVEL; dark reddish bro gravel to 3" in diameter, scatted) O' - 12': GRAVEL and COBBLES with silf moist, loose to medium dense gravelly CLAY layer at 7', about and andesite gravel and cobbit to medium- grained sand materials.	red cobbles [Colluvium] by sand matrix; dark redd by massive to stratified wit strate to subang strate sand bould strate small bould	ish brown, slightly moist to h 4" thick dark brown gular sandstone, siltstone,		No Samples Collected
ote: Soil Classifications (USCS) based on fi	eld descriptions			
	① Sandy Gravel			
	3 Gravel and		Gravelly Clay	



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Geotechnical Engineering
Geology

TEST_HOLE_LOG2 MBR02.GPJ WOMACK.GDT 11/7/02

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TEST HOLE LOG

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PRO.	JECT	NA.	ME:	Moor	nlight E	Basin Ra	anch	₩-1						D,	ATE:	9-5-02		·
PROJ	JECT	LO	CAT	ION:	Cowbo	y Heav	en Pha	use 3-B		- · · · · · · · · · · · · · · · · · · ·		·		Н	OLE N	Ю.: В	H02-7	
TEST	. HOT	E L	OC/	ATION:	Appr	oximate	ly betv	veen Lots 46 and 47	of Cowboy H	eaven Phase 3	Develop	ment						
ELEV.	ATIO	N G	i.S.	(ft.): ~	7828	то	TAL D	EPTH (ft.): 50	GROUNDW	VATER LEVEL ((ft.): 29		MEAS	URE	D FR	OM: (Ground	Surface
RILL	TYF	E:	CM	E 850		НАМ	MER:	140 # Automatic	DRILL CO:	HazTech Drilli	ng	DRILL	.ER: :	Sam		LO	GGED	BY: gs
DEPTH (ft.)	GRAPHICAL LOG		SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	COMMENTS: Dri Sample using 2" s 140 pound CME a	DESCF II with 8 1/2" olit-spoon sa utohammer	RIPTION OD, 4 1/4" ID h mpler without lir	ollow ste	em auge /-rods, :	ers. and LSIOW	CONTENT (%)	DRY DENSITY (PCF)	LIQUID	PLASTICITY INDEX (%)	WELL COMPLETION
		1	X	15	50			0.0-12.0ft Clayey yellowish brown, s massive, subangu [Landslide Debris	to Silty Sand slightly moist lar siltstone	dy GRAVEL with to very moist, n	h cobble: nedium d	s; dense,						
		2	M	15	35			12.0-24.0ft Gravel gray, slightly moist about 25% subang medium- to coarse	to moist, stif ular to subro	if to very stiff, plunded gravel to	astic, ma 3/4", 25	assive,						
		3	X	13	55			[Landslide Debris]									,	
		4		16	35				.	· · · · · · · · · · · · · · · · · · ·								
		5	<u>X</u>	30	55			24.0-38.0ft Clayey at 29', medium dens subangualr shale, s to 1 1/2", 35% medi fines [Landslide Del	se to dense, iltstone, sand um- to coars	massive about 4 dstone, and and	40% lesite gra	vel						



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29	PROJECT	LOCA	TION: (Cowbo	y Heave	n Pha	se 3-B	Н	OLE NO	D.: BH	102-7	
29	DEPTH (ft.) GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	DESCRIPTION	MOISTURE CONTENT (%)	DRY DENSITY (PCF)	LIQUID LIMITS (%)	PLASTICITY INDEX (%)	WEII
39	30 31 32 33 34 35 36 37											
48————————————————————————————————————	39 40 41 42 43 44 45 46						42.0-50.0ft SHALE; very dark gray, dry to slightly moist, weak, faintly laminated, extremely fractured, some red-brown mottling [Albino, Muddy, and Thermopolis Formation					
	48 49 50	10	50/3"	100								



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TEST HOLE LOG

PAGE 1 OF 2

Geology PROJECT NAME: Moonlight Basin Ranch DATE: 9-6-02 PROJECT LOCATION: Cowboy Heaven Phase 3-B HOLE NO.: BH02-8 TEST HOLE LOCATION: Approximately between Lots 47 and 48 of Cowboy Heaven Phase 3 Development ELEVATION G.S. (ft.): ~7820 TOTAL DEPTH (ft.): 55 GROUNDWATER LEVEL (ft.): 24 MEASURED FROM: Ground Surface DRILL TYPE: CME 850 HAMMER: 140 # Automatic DRILL CO: HazTech Drilling DRILLER: Sam LOGGED BY: gsv UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION MOISTURE CONTENT (%) S.P.T. (N) BLOWS/FT. DEPTH (ft.) PLASTICITY LIQUID LIMITS (%) COMMENTS: Drill with 8 1/2" OD, 4 1/4" ID hollow stem augers. SAMPLE Sample using 2" split-spoon sampler without liners, NW-rods, and WELL COMPLE INDEX (140 pound CME autohammer 0.0-29.0ft Silty to Clayey Sandy GRAVEL with cobbles; brown, slightly moist to moist, loose to medium dense, massive, siltstone, sandstone, and shale gravel [Landslide Debris] 8 30 8 55 7 30 1 29 30 20 21 22 19 0



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PRO.	PROJECT NAME: Moonlight Basin Ranch							DATE: 9-6-02							
PRO	PROJECT LOCATION: Cowboy Heaven Phase 3-B								HOLE NO.: BH02-8						
DEPTH (ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	DESCRIPTION	MOISTURE	CONTENT (%) DRY	DENSITY (PCF)	LIMITS (%)	PLASTICITY INDEX (%)	WELL		
29		2 3	20	100			29.0-39.0ft Gravelly to Sandy CLAY; light brown to light gray, moist to very moist, very stiff, plastic (appears bentonitic), massive, some red-brown oxidation mottling [Landslide Debris]								
36 37 38 39 39 31 40 31 41 41 41 41 41 41 41 41 41 41 41 41 41		4 \	59	40			39.0-44.0ft Clayey Sandy GRAVEL and COBBLES; brown, very moist, very dense, massive, angular broken sandstone gravel to 1 1/2" in diameter [Landslide Debris]								
14 15 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	0:0	5	56	100			44.0-49.0ft CLAY; dark brownish gray, moist, hard, massive [Residual Shale]								
19 1 2 3 3 4 4 1 1				100			49.0-55.0ft SHALE; very dark gray, slightly moist, very weak, laminated to stratified with 1-3" layers of very fine sandy siltstone, closely to extremely fractured, weathered [Albino, Muddy, and Thermopolis Formation undifferentiated]								
5	-														



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TEST HOLE LOG

PAGE 1 OF 2

Geotechnical Engineering Geology PROJECT NAME: Moonlight Basin Ranch DATE: 9-6-02 PROJECT LOCATION: Cowboy Heaven Phase 3-B HOLE NO.: BH02-9 TEST HOLE LOCATION: Approximately in the west-central portion of Lot 49, Cowboy Heaven Phase 3 Development ELEVATION G.S. (ft.): ~7830 TOTAL DEPTH (ft.): 45.5 GROUNDWATER LEVEL (ft.): 34 MEASURED FROM: Ground Surface DRILL TYPE: CME 850 HAMMER: 140 # Automatic DRILLER: Sam DRILL CO: HazTech Drilling LOGGED BY: gsv UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION MOISTURE CONTENT (%) WELL COMPLETION S.P.T. (N) BLOWS/FT. DEPTH (ft.) LIQUID LIMITS (%) PLASTICITY COMMENTS: Drill with 8 1/2" OD, 4 1/4" ID hollow stem augers. INDEX (%) SAMPLE Sample using 2" split-spoon sampler without liners, NW-rods, and 140 pound CME autohammer 0.0-10.0ft Sandy CLAY with scattered gravel; light brown to reddish brown, slightly moist, stiff, massive, angular sandstone gravel [Colluvium] 14 1 80 37 100 10.0-24.0ft Clayey Sandy GRAVEL; dark brown to brown to gray, slightly moist to moist, medium dense to dense, massive, angular shale, siltstone, and sandstone gravel [Landslide Debris] 3 29 4 37 75 20 21 22 23 24.0-29.0ft Sandy GRAVEL; dark gray, dry, medium dense, 5 40 massive, mostly angular siltstone gravel [Landslide Debris]



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TEST HOLE LOG

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Geotechnical Engineering Geology PROJECT NAME: Moonlight Basin Ranch DATE: 9-6-02 PROJECT LOCATION: Cowboy Heaven Phase 3-B HOLE NO.: BH02-9 GRAPHICAL LOG UNCONFINED STRENGTH (TSF) CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION WELL COMPLETION CONTENT (%) S.P.T. (N) BLOWS/FT. DEPTH (ft.) MOISTURE LIQUID LIMITS (%) SAMPLE INDEX 29.0-39.0ft Clayey to Silty Sandy GRAVEL; brown and dark gray, moist to wet at 34', medium dense, massive, angular shale gravel [Landslide Debris] 16 100 33 16 80 38 50/6" 39.0-45.5ft SHALE; very dark gray, dry to very moist at contact with gravel, very weak, laminated, extremely fractured, moderately to highly weathered, some reddish brown oxidation mottling [Albino, Muddy, and Thermopolis Formation undifferentiated] 9 60 100 46 51 52 53 54 55



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Geotechnical Engineering Geology

MBR02.GPJ WOMACK.GDT

PROJECT NAME: Moonlight Basin Ranch DATE: 9-6-02 PROJECT LOCATION: Cowboy Heaven Phase 3-B HOLE NO.: BH02-10 TEST HOLE LOCATION: Approximately in the southwest corner of Lot 26, Cowboy Heaven Phase 3 developement TOTAL DEPTH (ft.): 30.5 MEASURED FROM: na ELEVATION G.S. (ft.): ~7840 GROUNDWATER LEVEL (ft.): na DRILLER: Sam DRILL TYPE: CME 850 HAMMER: 140 # Automatic DRILL CO: HazTech Drilling LOGGED BY: gsv UNCONFINED STRENGTH (TSF) GRAPHICAL LOG CLASSIFICATION RECOVERY (%) DRY DENSITY (PCF) DESCRIPTION CONTENT (%) ETION S.P.T. (N) BLOWS/FT. PLASTICITY INDEX (%) MOISTURE LIQUID LIMITS (%) DEPTH (ft.) COMMENTS: Drill with 8 1/2" OD, 4 1/4" ID hollow stem augers. SAMPLE Sample using 2" split-spoon sampler without liners, NW-rods, and 140 pound CME autohammer WELL COMPLE 0.0-4.0ft Sandy GRAVEL and COBBLES; light brown, dry, loose, massive, sandstone and andesite clasts [Colluvium] 4.0-24.0ft Clayey to Sandy GRAVEL; monolithologic (shale 53 100 gravel), dark brownish gray, dry to slightly moist, medium dense to dense, massive, angular shale gravel with pockets of gray-brown sandy clay [Landslide Debris] 35 75 3 31 65 18 28 100 4 20 21 22 23 24.0-29.0ft CLAY; brownish gray, moist, stiff to very stiff, 5 15 100 massive, some reddish brown mottling [Residual Shale] 26



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TEST HOLE LOG

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PRO	JECT	NAME	: Moon	light B	asin Ra	nch			D/	ATE:	9-6 - 02		
PROJ	JECT I	LOCAT	TION: (Cowbo	y Heave	n Pha	se 3-B		НС	DLE N	Э.: В	H02-10	
DEPTH (ft.)	GRAPHICAL LOG	SAMPLE	S.P.T. (N) BLOWS/FT.	RECOVERY (%)	UNCONFINED STRENGTH (TSF)	CLASSIFICATION	DESCRIPTION	MOISTIBE	CONTENT (%)	DRY DENSITY (PCF)	LIQUID IMITS (%)	PLASTICITY INDEX (%)	WELL
9		6	61	100			29.0-30.5ft SHALE; very dark brownish gray, slightly moist, weak, laminated, extremely fractured, moderately weathered [Albino, Muddy, and Thermopolis Formnation undifferentiated]						